## RFP ADDENDUM #3

## Date of Addendum: 03/05/2024

## NOTICE TO ALL POTENTIAL RESPONDENTS

The Request for Proposals (RFP) is modified as set forth in this Addendum. The original RFP Documents will remain in full force and effect, except as modified by this Addendum, which is hereby made part of the RFP. Respondent shall take this Addendum into consideration when preparing and submitting its Proposal.

## 1. PROPOSAL SUBMITTAL DEADLINE

The Amended Proposal submittal deadline remains the same and is not changed by this Addendum.

### 2. QUESTIONS AND ANSWERS

The following questions and answers are provided as a matter of information to clarify issues raised about the RFP. To the extent that changes to the RFP are required based on the questions received, the RFP has been modified as noted above in the RFP section of this Addendum.

Item	Questions and Answers
3.1	<u>Question</u> : Please confirm the scope of water and fire suppression utility services. Is the contractor to assume just extension of existing services to A, G and H floats, or are upgrades on the headwalk and supply lines anticipated? Can you provide as-built information on the existing utilities? <u>Answer</u> : The Contractor is expected to tie into the existing services. Please find attached the as-builts as requested.
3.2	<u>Question:</u> We request any information and pictures especially you might have on the old floats or debris pile you want removed that you may have? Also is there a clear path or right of way where we could bring in a ramp type barge to the nearest beach to haul them out of there via water? <u>Answer</u> : Beach access will be available starting September 2024. Pictures of the debris to be removed ("legacy floats") are attached below. Additionally, there is a clear right-of-way after September 15th and before May 15th, weather permitting.
3.3	<u>Question:</u> Please provide the quantity of legacy wood pilings that will be replaced with steel piling noted in section 1.1 B, page 6. Please provide a map denoting the locations of all legacy wood pilings to be replaced. <u>Answer:</u> Further information on legacy wood pilings, including a map of the locations, are attached below. There are a total of 153 wood piles and 24 steel piles.
3.4	<u>Question</u> : Can you confirm the grants for this project don't require the Buy American Act to be applicable for this project? <u>Answer</u> : This project does not have the Buy America Act requirements.

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3.5	Question: Please confirm the vessel parameters to be used for design (vessel lengths, beam widths, weight, etc.) of the various slips and side tie moorage. Are overlength vessels to be assumed for design of finger floats? Answer: Our current practice is 10% overall finger length. A Float does not have vessel parameters. A Float has one landing craft that is 85ft overall length permanently assigned. A Float historically will moor large fishing vessels up to 60-85ft in length and often more 3-4 abreast on the outside (west side) and 2-3 wide on the inside (east side) of the float. G Float vessel parameters are 0' – 29' and H Float vessel parameters are also 0' - 29', so the stalls are currently 24' fingers. G and H floats are currently 21' apart with an assumed vessel width of 10'.
3.6	Question: Please confirm the snow load to be used for design of the floatation/freeboard. The specified 100psf snow load and requirement for DL+snow load freeboard to be at least 6" will result in floats with high freeboard. The snow load requirements are also significantly greater than the specified live load DL+live load freeboard requirements. <u>Answer</u> : With no snow load, the current floats are showing an 18" freeboard. Please match existing design.
3.7	Question: Appendix D 4.1.1 Q: Will HDG Steel cleats or Aluminum cleats be allowed? Answer: HDG Steel Cleats are preferred
3.8	Question: Appendix D 4.1.9 C: Existing concrete float chaseways consist of 5.5" x 16.5" clear space. Will this same type be acceptable? Answer: Yes.
3.9	Question: Appendix D 4.2.1 K: Existing concrete float system utilizes waler and through rods. Will this same type be acceptable? Answer: Yes.
3.10	Several questions were received regarding Appendix D, Section 4.2.2. This section has been stricken from the Request for Proposal documents.
3.11	<u>Question</u> : Appendix D 4.2.3: Is FRP (fiber reinforced polymer) structural reinforcement acceptable? Is epoxy coated bar accepted as a one-to-one replacement over galvanized rebar reinforcement? <u>Answer</u> : Yes.
3.12	<u>Question</u> : Appendix D 4.2.4 A: Is an ISO 9001 Plant Certification acceptable in lieu of PCI Certification? <u>Answer</u> : Yes.
3.13	<u>Question</u> : Appendix D 4.2.5: Existing concrete float system provides all cleats bolted through timber walers. Is this acceptable? <u>Answer</u> : Yes.
3.14	Question: Appendix D 4.2.9 K: Are galvanized through rods accepted? Are FRP walers accepted?

	Answer: Yes to both. We expect contractor to make these decisions based on cost.
3.15	<u>Question</u> : Appendix D: Specification section 4.2.1 items C and K, and 4.2.9 items I, J, K appear to be in conflict. Can you please clarify if a modular concrete system with a through rod and waler structural support system is permitted on this project? <u>Answer</u> : Yes.
3.16	<u>Question</u> : Appendix D 3.5: The RFP specification is for a 100 PSF snow load requires floats to be designed to a 26" freeboard level to accommodate a nearly 10-foot-deep snow event on the dock floatation system. Existing concrete float systems on site have likely been designed at a 70 PSF snow load and 20" freeboard or a 60 PSF snow load and 18" freeboard. Please clarify and confirm all freeboard levels for docks under DL, DL + LL, DL + SL, DL + LL + SL. <u>Answer</u> : With no snow load, the current floats are showing an 18" freeboard. Please match existing design.
3.17	<u>Question</u> : Appendix D: 4.1.6: Subsection B states that creosote treatment is not allowed, unless approved by the Owner. Will creosote treatment be allowed for glulam members located above water and not exposed to surface contact on trestles and floats? Additionally, many treatment alternative allowed by AWPA are prohibited by regulatory agencies for over/near water applications. Can you confirm the specific pressure treatments that will be permitted by regulatory agencies for this project? <u>Answer</u> : Yes.
3.18	Question: Appendix D 4.1.3: Will AWPA Marine Grade No. 1 Southern Pine be accepted as an equal? Answer: Yes.
3.19	<u>Question</u> : RFP Sections 1.2.A references replacement of the float systems for A/G/H floats and Section 1.2.C states "Design and installation of the Utility Service upgrades, including power, lighting, water, and fire suppression to all applicable areas in the float system and related upland improvements." Section 3.9 <i>Utilities</i> , second paragraph states "The electrical systems will include power to all slips on floats A and G, LED lighting, and all ancillary elements of the project." Lastly, the Appendix D Part 4.6.4 table only shows pedestal receptacle requirements for A and G floats. Please verify if power and lighting is required for H-float as well, and if so, please confirm receptacle requirements. <u>Answer</u> : H Float will not be powered.
3.20	<u>Question</u> : Appendix D part 4.6.2 <i>Service</i> requires new 208/120V, 3-phase electrical service for the harbor located in the uplands near the top of the Gangway. It is our understanding that the existing harbor service is 600A, 480V, 3-phase and feeds a distribution panel and substations on the dock that require 480V. We believe the distribution panel and substations were installed in 2015 and the service equipment was modified at the same time to add in new feeders and an Automatic Transfer Switch. Is a new 208/120V, 3-phase service truly required for this project, or can the existing 480V, 3-phase service and recently installed dock distribution panel be utilized to feed the electrical systems for floats A, G, and H, provided sufficient capacity exists within the equipment? <u>Answer</u> : Yes, existing will suffice.

3.21	<u>Question</u> : Appendix D part 4.6.4 requires pedestals "with a utility meter". We don't believe the other pedestals on the remaining floats have utility (Chugach Electric Association) meters, only kWh meters integral to the pedestals. Please confirm CEA meters are not required within the pedestals. <u>Answer</u> : CEA meter is not required.
3.22	<u>Question</u> : Appendix D part 4.6.5 Lighting states to "provide luminaries atop select shore-tie pedestals on the floating docks as needed for the specified amount of illumination". Is the desired intent to provide the pedestals with integral pedestal top lighting for all dock illumination, or are pole mounted LEDs spaced throughout the docks more desirable? The photocell and hand-off-auto lighting controls requirement within Part 4.6.3.G eludes to the separate pole mounted LED lighting. Please clarify the intent. <u>Answer</u> : Design-Builder to determine based on cost; either are acceptable.
3.23	<u>Question</u> : Is the Owner/Obligee requiring a P&P Bond for six years after the performance period? (what is the length of the performance period? Second, when will the Owner/Obligee require the winning bidder to provide the performance and payment bonds? <u>Answer</u> : With respect to Article 5.1 A. Please change to read "These Bonds shall remain in effect for a minimum of one year after the date when the final payment becomes due, or when construction is complete, whichever is later, except as provided otherwise by Laws or Regulations." The purpose of this change is to serve as a warranty bond to ensure completion of the project. The winning bidder shall provide the performance and payment bond prior to the start of construction. With respect to Bonds and Insurance, Article 5.1 C. Please strike this entire section.
3.24	<u>Question</u> : In regards to the warranty, wouldn't some of the large material items involved already come with longer term manufacturers warranties (so the warranty obligation essentially passes through the contractor)? <u>Answer</u> : We would rely on the manufacturer's warranties in those cases, yes.
3.25	<u>Question</u> : As referenced in Appendix A 2.D., please provide all reports, tests, and drawings associated with the representation being made by submission of this proposal. <u>Answer</u> : Please see the attached "Whittier Small Boat Harbor Improvements – Phase I" for this data.

## END OF ADDENDUM

## CITY OF WHITTIER REPLACEMENT FLOAT PHASE 1 Whittier, AK

REV.	DRAWING:	SHEET NO.	DRAWING TITLE	DESCRIPTION
	C1.0		COVER SHEET	
	S1.0		ABBREVIATIONS & LEGEND	
	OL1.0	1	OVERALL LAYOUT	
	DL1.0	2	DIMENSIONAL LAYOUT	
	FL1.0	3	FLOAT LAYOUT	
	UL1.0	4	UTILITY LAYOUT	ELECTRICAL
	UL2.0	5	UTILITY LAYOUT	MECHANICAL
	CL1.0	6	CLEAT LAYOUT	
	SL1.0	7	MISC. MATERIAL FABRICATION	
	AL1.0-AL1.2	8-10	ASSEMBLY LAYOUT	Z DOCK MARGINAL (60.776245, -1
	AL2.0-AL2.1	11-12	ASSEMBLY LAYOUT	F DOCK MAINWALK
	AL3.0-AL3.1	13-14	ASSEMBLY LAYOUT	E DOCK MAINWALK
	AL4.0-AL4.1	15-16	ASSEMBLY LAYOUT	D DOCK MAINWALK
	AL5.0	17	ASSEMBLY LAYOUT	C-D DOCK MARGINAL
	AL6-AL6.2	18-20	ASSEMBLY LAYOUT	FINGERS
	FS1.0	21	FLOAT SECTIONS	
	SA1.0-SA1.1	22-23	SUB-ASSEMBLY DETAILS	GENERAL DETAIL
	SA2.0	24	SUB-ASSEMBLY DETAILS	TRI-FRAME
	SA2.1	25	SUB-ASSEMBLY DETAILS	END/SIDE PILE GUIDE
	SA2.2	26	SUB-ASSEMBLY DETAILS	TRANSFORMER City of V
	SA2.3	27	SUB-ASSEMBLY DETAILS	TRANSITION PLATES Existing

Bellingham MARINE RARINE Ballingham Karline Rake Wa 8228 TEL: (360) 380-2142 FAX: (360) 384-8134	JOB NUMBER: 14-1665	SCALE: DATE:	N.T.S. 03/05/15	CITY OF WHITTIER
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### VICINITY MAP:



SITE MAP:



	A	В		C	D		E
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ABBREVIATIO	NS DESCRIPTION	ABBREVIA	ATIONS	DESCRIPTION	ABBREV	IATIONS	DESCRIP
L	ANGLE	P.L.	••	PLATE	C	.В.	CARRIAG
Ø	SQUARE	PLC'S	'S	PLACES	E	В.	EYE BOLT
B.F.	BOARD FEET	P.S.I	. <b>I</b> .	POUNDS PER SQUARE INCH	F.	W.	FLAT (CU
BOTT.	BOTTOM	PT		POST TENSIONING	F.H.	W.S.	FLAT HEA
CHAMF.	CHAMFER	P.V.C	C.	POLYVINYL CHLORIDE	Н	В.	HEX BOLT
CHKR.	CHECKER	P.W	1.	POTABLE WATER	H.C	C.N.	HEX COU
Ę	CENTER LINE	QTY	<b>Y</b> .	QUANTITY	H.H	Н.В.	HEAVY HE
CLR.	CLEAR	R.		RADIUS	Н	N.	HEX NUT
CONC.	CONCRETE	REF	=	REFERENCE	J.	N.	JAM NUT
CORN.	CORNER	REQ'	l'D	REQUIRED	L	В.	LAG BOLT
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F.F.		THRE	D.	THREAD	P.P.	H.S.	PHILLIPS
F.B.	FREE BOARD/FLAT BAR	TRANS	SF.	TRANSFORMER			
FLT.	FLOAT	T.S.	·•	TUBE STEEL			
FL.	FULL LENGTH	TYP	۰ <u>.</u>	TYPICAL			
FT.	FOOT/FEET	U.H.M.	I.W.	ULTRA HIGH MOLECULAR WEIGHT (POLYETHYLEN	E)		
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GA.	GAUGE	W.	, ,	WIDTH			
GALV.	GALVANIZED	W.W.I	.F. '	WELDED WIRE FABRIC		$\widehat{\mathbf{A}}$	
H.D.	HEAVY DUTY	1.W.W	.M.	WELDED WIRE MESH		$\odot$	
H.D.P.E.	HIGH DENSITY POLYETHYLENE				DOCK	DESIGNATION	-
HORIZ.	HORIZONTAL						
HSS.	HOLLOW STRUCTURAL SECTIONS						
I.D.	INSIDE DIMENSION						
L.	LENGTH					SECTION	
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ABBREVIATIONS & LEGI	- OF -	SHEET NO.: - (	- ѕн	precision engineered floatation systems	

END



HEX BOLT IUT UT OLT WASHER INE SCREW WASHER R PLATE RE PLATE WASHER TAPPING SCREW RING R BOLT ROD IEAD HEX DRIVE SCREW IPS PAN HEAD SCREW

RIPTION AGE BOLT OLT CUT) WASHER HEAD WOOD SCREW BOLT COUPLER NUT

























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## \_AYOUT

## /HITTIER \_OATS PHASE 1

\_ 38-W4 (G HOLE FACES FLOAT & DAP END AS NECESSARY FOR POLYMER TO SIT FLUSH WITH DECK)

2x4 x 8" LONG SPACERS OR 2x8 CONTINUOUS BOARD WITH M HOLES FOR T.R. CLEARANCE NAILED ON W/ 16d NAILS @ 3/4" BELOW DECK

CROSS-OVER

MECH. UTILITY

P-10

P-09 (FIELD FAB. - P-11 TO CLEAR PILE)



S)	DESCRIPTION
).75" T.R. (C)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (Walkway)
.25" T.R. (D)	3/4"dia. x 7'-1 1/4" Long Thru-Rod (Walkway)

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		F DOCK MAINWALK	<					
		PLAN VIEW						
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						ROD(S)	DESCRIPTION 3/4"dia_x 7'-0.3/4" Long Thru-Rod (M	Valkway)
4						0.750"dia. x 7'-1.25" T.R. (D)	3/4"dia. x 7'-1 1/4" Long Thru-Rod (W	valkway)
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)	DESCRIPTION
75" T.R. (C)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (Walkway)
25" T.R. (D)	3/4"dia. x 7'-1 1/4" Long Thru-Rod (Walkway)



DESCRIPTION	
'-0 3/4" Long Thru-Rod (Walkway)	
'-1 1/4" Long Thru-Rod (Walkway)	



S)	DESCRIPTION
).75" T.R. (A)	3/4"dia. x 9'-0 3/4" Long Thru-Rod (Marginal)
25" T.R. (B)	3/4"dia. x 9'-1 1/4" Long Thru-Rod (Marginal)
).75" T.R. (C)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (Walkway)
	3/4"dia. x 7'-1 1/4" Long Thru-Rod (Walkway)



DESCRIPTION	
'-0 3/4" Long Thru-Rod (Walkway)	
'-1 1/4" Long Thru-Rod (Walkway)	



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-----REV. DATE:

ROD(S)	DESCRIPTION
0.750"dia. x 9'-0.75" T.R. (A)	3/4"dia. x 9'-0 3/4" Long Thru-Rod (Marginal)
0.750"dia. x 9'-1.25" T.R. (B)	3/4"dia. x 9'-1 1/4" Long Thru-Rod (Marginal)







-	-	Bellingham Northwest Division 5500 Nordic Way Ferndale, WA 98248	JOB NUME	BER:	SCALE:	1/8" = 1'-0"	CITY OF WHITTIER
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## S PHASE 1



	DESCRIPTION
R. (E)	3/4"dia. x 3'-1 3/4" Long Thru-Rod (Fingers)
R. (F)	3/4"dia. x 3'-5 1/2" Long Thru-Rod (Fingers)
R. (G)	3/4"dia. x 3'-8" Long Thru-Rod (Fingers)

38-F - CORNER BUMPER

12" H.D. CLEAT (6) PER FINGER

SA1

12" H.D. CLEAT (4) PER FINGER 48-8G 51. 6HT SA1.0 l **t**== - 38-E1 CORNER BUMPER 48-8G 2 SA1.0



	DESCRIPTION
R. (E)	3/4"dia. x 3'-1 3/4" Long Thru-Rod (Fingers)
R. (F)	3/4"dia. x 3'-5 1/2" Long Thru-Rod (Fingers)

GENERAL NOT 1. ALL THRU	ES: -RODS (T.R.) SHALL HAVE P.W. @ LUMBER ASSY. & F.W.	". @ STEEL ASSY. W/ L.W. & H.N. @ EA. END.	, <u> </u>				<u> </u>
					-	FIRST (6) T.R. SLEEVES @ TYP. T.R. SLEEVES @ 2-1/2" DOWN FROM DECK 3-3/4" DOWN FROM DECK	NOTE: REMOVE ALL HARDWARE, LUMBER & STEEL WELDMENTS. ROTATE FLOAT 180° FOR ASSY. AS SHOWN.
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N			2.0 TPHD-01 PP-01			G (FIELD ASSY.) 32' FINGER PLAN VIEW	
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		C PP-01		PLAN VIEW			
_							
4						ROD(S) 0.750"dia. x 3'-1.75" 0.750"dia. x 3'-5.50"	DESCRIPTION           .R. (E)         3/4"dia. x 3'-1 3/4" Long Thru-Rod (Fingers)           .R. (F)         3/4"dia. x 3'-5 1/2" Long Thru-Rod (Fingers)
	-	- Bellingham Northwest Division 5500 Nordic Way Fendale, WA 98248	JOB NUMBER:	SCALE:	1/4" = 1'-0"	CITY OF W	
	-	- MARINE (360) 380-2142 - FAX: (360) 384-8134	14-1665	DATE:	02/24/15		
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	02/22/15	DATE:	4-1665	MARINE (360) 380-2142 FAX: (360) 384-8134	-		-	-		
	T.A.P.	DRAWN BY:	ZE: 11x17	The World's Most Comprehensive Marina Builder	-	-	-	-		
Whittier, AK	E01.0		QUALS TO ONE INCH ON	This drawing contains proprietary information which is the property of Bellingham Marine Industries, Inc., and shall	-	-	-	-		
	FS1.0	DRAWING:	ING. ADJUST SCALES IF DISTANCE	not be copied, reproduced or made available to third arties without prior written permission from Bellingham	parties without prior written permission from Bellingham	parties without prior written permission from Bellingham	-	-	-	-
FLOAT SECTION	21			Marine Industries, Inc. UNIFLOAT®, UNIBOLT®, UNIDECK® and UNISTACK® are registered trademarks	-	-	-	-		
	21	SHEET NO	Ι	of © Bellingham Marine Industries, Inc.	BY:	DESCRIPTION	V. DATE:	REV		

## CTIONS



-3x8 WALERS

-2x8 RUB BOARD

FLOAT DECK







24

SHEET NO .:

-

-

DESCRIPTION

BY:

-

REV. DATE:

SUB-ASSEMBLY DETAILS



25

SHEET NO .:

REV. DATE:

DESCRIPTION

BY:

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## DETAILS





# CITY OF WHITTIER **REPLACEMENT FLOAT PHASE 2**

## Whittier, AK

REV.	DRAWING:	SHEET NO.	DRAWING TITLE	DESCRIPTION
	C1.0		COVER SHEET	
	S1.0		ABBREVIATIONS & LEGEND	
	OL1.0	1	OVERALL LAYOUT	
	DL1.0	2	DIMENSIONAL LAYOUT	
	FL1.0	3	FLOAT LAYOUT	
	UL1.0	4	UTILITY LAYOUT	ELECTRICAL
	UL2.0	5	UTILITY LAYOUT	MECHANICAL
	CL1.0	6	CLEAT LAYOUT	
	SL1.0	7	STEEL WELDMENT LAYOUT	
	AL1.0-AL1.1	8-9	ASSEMBLY LAYOUT	X DOCK MARGINAL
	AL2.0-AL2.2	10-12	ASSEMBLY LAYOUT	B & C DOCK MAINWALK
	AL3.0-AL3.1	13-14	ASSEMBLY LAYOUT	FINGERS
	FS1.0	15	FLOAT SECTIONS	
	SA1.0	16	SUB-ASSEMBLY DETAILS	GENERAL DETAIL
	SA2.0	17	SUB-ASSEMBLY DETAILS	TRI-FRAME
	SA2.1	18	SUB-ASSEMBLY DETAILS	END/SIDE PILE GUIDE
	SA2.2	19	SUB-ASSEMBLY DETAILS	TRANSFORMER
	SA2.3	20	SUB-ASSEMBLY DETAILS	DUMMY TRI-FRAME
	SA2.4	21	SUB-ASSEMBLY DETAILS	MINI-FRAME

(60.776245, -148.684982)

City of Whittier Existing Marina

Bellingham 5500 Nordic Way Ferndale, WA 98248	JOB NUMBER:	SCALE:	N.T.S.	CITY OF WHITTIER
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VICINITY MAP:





**AS BUILT** 08-25-17

DRAWING STATUS:

TYP. DRAWING	DECODIDITION	TYP. DRAWING		TYP. HARDWARE	
ADDREVIATIONS					
L		P.L.		С.В.	
		PLUS		E.D.	
B.F.	BOARD FEET	P.S.I.	POUNDS PER SQUARE INCH	F.W.	
				F.H.W.S.	
		P.V.C.		H.B.	
CHKR.		P.W.		H.C.N.	
۴ مرب		QIY.	QUANTITY	H.H.B.	
CLR.	CLEAR	R.	RADIUS	H.N.	
CONC.	CONCRETE	REF.	REFERENCE	J.N.	
CORN.	CORNER	REQ'D	REQUIRED	L.B.	
CSC.	COUNTERSINK	REV.	REVISION	L.W.	LOCK WASHER
CU.	CUBIC	S.B.O.	SUPPLIED BY OTHERS	M.S.	MACHINE SCREW
D.	DEPTH	SCH.	SCHEDULE	P.W.	PLATE WASHER
Ø	DIAMETER	SHT.	SHEET	S.P.	SHEAR PLATE
DET.	DETAIL	SIM.	SIMILAR	S.P.W	SQUARE PLATE WAS
DBL.	DOUBLE	SPA.	SPACES	S.T.S.	SELF TAPPING SCRE
DWG.	DRAWING	S.S.	STAINLESS STEEL	S.R.	SPLIT RING
EA.	EACH	STD.	STANDARD	Т.В.	TIMBER BOLT
E.W.	EACH WAY	T&B	TOP AND BOTTOM	T.R.	THRU-ROD
EPS	EXPANDED POLYSTYRENE	T.B.D.	TO BE DETERMINED	F.H.H.D.S.	FLAT HEAD HEX DRIN
F.F.	FAR FACE	THRD.	THREAD	P.P.H.S.	PHILLIPS PAN HEAD
F.B.	FREE BOARD/FLAT BAR	TRANSF.	TRANSFORMER		
FLT.	FLOAT	T.S.	TUBE STEEL		
FL.	FULL LENGTH	TYP.	TYPICAL		
FT.	FOOT/FEET	U.H.M.W.	ULTRA HIGH MOLECULAR WEIGHT (POLYETHYLENE)		
FW	FIRE WATER	VERT	VERTICAL	LEGEND	
GA	GAUGE	W	WIDTH		
GALV		W W F	WEI DED WIRE FABRIC		
нр		W/ W/ M	WELDED WIRE MESH	(A)	1
HDPE				DOCK DESIGNATION	FLOAT
					·
по <u>о</u> .	HOLLOW STRUCTURAL SECTIONS				
I.D.				SECTION	1
L.				DESIGNA	.HON
LG.				A ´	
L.L.H.				FS1.0	REVISIO
L.L.V.					
LONG.	LONGITUDINAL				JREF.
MIN.	MINIMUM				
N.F.	NEAR FACE				
NO.	NUMBER			I	(1)
N.T.S.	NOT TO SCALE			A	SA5.0
O.A.	OVERALL			<b>F</b> S1.0	
O.C.	ON CENTER				*
O.D.	OUTSIDE DIMENSION				
OPP.	OPPOSITE			SECTION CALL-OUT	SUB-ASSY. SEC
ellingham	JOB NUMBER:	SCALE: N.T.S.			

Bellingham 5500 Nordlc Way Ferndale, WA 98248	JOB NUMBER:		SCALE:	N.T.S.	CITY OF WHI
MARINE FAX: (360) 380-2142 FAX: (360) 384-8134	17-1711		DATE:	02/17/2017	
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IEX BOLT

AD WOOD SCREW JPLER NUT



SHEET NO .:

REV. DATE:

DESCRIPTION

BY:

1






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FL1.0

3

FLOAT	LAYC

## OUT

5/6T FLOAT FOR RELOCATED A-B-C TRANSFORMER









-	- - - -		Bellingham MARINE Northwest Division 500 Nordic Way Ferndale, WA 88248 TEL: (360) 380-2142 FAX: (360) 384-8134	JOB NUMBER: 17-1711	SCALE: DATE:	N.T.S. 02/17/2017	
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## OUT

# HITTIER OATS PHASE 2





E F	
D(S)	DESCRIPTION
'-0.75" T.R. (F)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (B & C Dock Walkway)
'-1.50" T.R. (G)	3/4"dia. x 7'-1 1/2" Long Thru-Rod (B Dock Alternate @ UH-01)
'-0.75" T.R. (H)	3/4"dia. x 9'-0 3/4" Long Thru-Rod (X Dock Walkway)
x 7'-3.00" T.R. (I)	A449 3/4"dia. x 7'-3" Long Thru-Rod (B & C Dock @ MF-01)
2'-4.50" T.R. (J)	3/4"dia. x 12-4 1/2" Long Thru-Rod (C Dock to 5x6T Transformer Float)



SHEET NO .:

REV. DATE:

DESCRIPTION

BY:

9







	DESCRIPTION
75" T.R. (F)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (B & C Dock Walkway)
50" T.R. (G)	3/4"dia. x 7'-11/2" Long Thru-Rod (B Dock Alternate @ UH-01)
75" T.R. (H)	3/4"dia. x 9'-0 3/4" Long Thru-Rod (X Dock Walkway)
-3.00" T.R. (I)	A449 3/4"dia. x 7'-3" Long Thru-Rod (B & C Dock @ MF-01)
.50" T.R. (J)	3/4"dia. x 12'-4 1/2" Long Thru-Rod (C Dock to 5x6T Transformer Float)



-3-7/50 FINGER-SEE DWG, AL3.1

	DESCRIPTION
-)	3/4"dia. x 7'-0 3/4" Long Thru-Rod (B & C Dock Walkway)
5)	3/4"dia. x 7'-1 1/2" Long Thru-Rod (B Dock Alternate @ UH-01)
4)	3/4"dia. x 9'-0 3/4" Long Thru-Rod (X Dock Walkway)
. (1)	A449 3/4"dia. x 7'-3" Long Thru-Rod (B & C Dock @ MF-01)
1)	3/4"dia. x 12'-4 1/2" Long Thru-Rod (C Dock to 5x6T Transformer Float)





ROD(S)	DESCRIPTION
ia. x 3'-1.75" T.R. (A)	3/4"dia. x 3'-1 3/4" Long Thru-Rod (2-7 Fingers)
ia. x 3'-5.50" T.R. (B)	3/4"dia. x 3'-5 1/2" Long Thru-Rod (2-7x24 Fingers)
ia. x 4'-1.75" T.R. (C)	3/4"dia. x 4'-1 3/4" Long Thru-Rod (3-7 Fingers)
)"dia. x 3'-4.75" T.R. (D)	A449 3/4"dia. x 3'-4 3/4" Long Thru-Rod (2-7 Finger @ MF-01)
0"dia. x 4'-4.75" T.R. (E)	A449 3/4"dia. x 4'-4 3/4" Long Thru-Rod (3-7 Finger @ MF-01)

+	Α Ι	В	С		D	1	E	
							12" H.D. CLEAT	
		_ 2x4 TOP 38	$-W^2$ CONTROL $= \frac{D}{E \leq 1.0}$ 4	8-18G –		∕─ 48 <b>-</b> 16GD	(0) TERTINOER	
	P	16 - D BOARD	JOINT					
	F-							
	MF-	-01 DF-2 & DF-2A			3	-7/48		
					<u>c</u>		<u>c</u>	
-	L							
			3	227 St. Carl				
	ISA2.		,   _		$\mathbf{A}$			
			DER $\mathbf{b}$ $\frac{\mathbf{b}}{\mathrm{FS1.0}}$ 4	8-18G —		~ 48-16GD	2	
			ALL AROUND)		SALO	54	<b>V</b>	
	SA2.			$\frown$				
				(1)	3-7 x 48'	FINGER		
	DF				/ PLAN	VIEW		
_		~						
					12" H	D. CLEAT		
	2x4 TOP ~ 38-W2			- 49 1600	(8) PI	ER FINGER		
	► BOARD	JOINT		- 48-10GD				
P-16		· · · · · · · · · · · · · · · · · · ·						
~ /								
MF-01-	——————————————————————————————————————		3-7	/50				
l l					С			
_L-01				C I				
								EPG-03 or EP
SA2.4								(
	FENDER	► <u>D</u> FS1.0 48-18G -∕		─_ 48-16GD				
		AROUND)	SALU		SAI.U			
SA2.3					·			
-								
DF-3 -/	DF-5		$3-7 \times 50$	)' FINGER				
			PLAI	N VIEW				
	SS TEK SCREW							
	FLOAT DECK-							
	CAST-IN	P-16 POLYMER COVER						
	3/4"Ø H.B. W/ COUPLER							
m								
	3/4"Ø x 1	3/4" LONG H.B						
_		ALL				CENERAL NOTES		
						1. ALL THRU-RODS (T.R.) SHALL HAV	/E P.W. @ LUMBER ASSY. & F.W. @ STEEL ASS	Y. W/ L.W. & H.N. @ EA. END
	X					. ,	~	- 1
	Ň						DESCRIPTION	
	$\frown$					0.750"dia. x 3'-1.75" T.R. (A)	3/4"dia. x 3'-1 3/4" Long Thru-Roc	a (2-7 Fingers)
	A 3-7 FINGER EN	D				0.750 ula. x 3 - 5.50° I.K. (B)	5/4 uid. X 5 -5 1/2 Long Inru-Kod ( 3/4"dia x 4'-1 3/4" Long Thru-Poo	2-7X24 Filigers)
	- ASSY DETAIL	-				A449 0.750 dia. x 4-1.75 T.R. (C)	A449 3/4"dja x 3'-4 3/4" Long Thru-Rod /	(2-7 Finger @ MF-01)
		-					Δ449 3/4"dia × 4"-4 3/4" Long Thru Rod (	3-7 Finger @ ME_01)
·	<u>-</u>	- Bollingham Northwest Division						
	-	5500 Nordic Way Ferndale, WA 98248	JOR NOMBER:	SCALE:	1/4" = 1'-0"	CIT	Y OF WHITTIFR	
	-	- MARINE	17_1711		02/21/2017	011		
-		- The World's Most Comprehensive			UZIZ IIZUTI		IENT FLOATS PI	HASE 2
	-	Marina Builder	PAPER SIZE: 11x17	DRAWN BY:	С. М. К.			
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		or e demingham Manne Industries, Ind.		1				







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## FLOAT SECTIONS





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17

## SUB-ASSEMBLY DETAILS



SHEET NO .:

REV. DATE:

DESCRIPTION

BY:

SUB-ASSEMBLY DETAILS

18



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## **BLY DETAILS**



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## Item 3.2 Debris for Removal

Legacy floats (B/C/D/E/F) and headwalk (Y/Z) = 2300 linear feet x 7/8ft wide

Fingers	10ea x 481	4801t 400ft
Fingers	1700 x 22ft	4001t
Fingers	2000 x 20ft	900ft
Fingers	1200 x 20ft	265ft
Finders	2602 x 24ft	625ft
Total	2068 X 2411	3330 lft x 2/3ft wide





- Existing Steel Pile
  - = Existing Wood Pile



A Float – 18 Wood Piles



B Float – 14 Wood Piles 2 Steel Piles



C Float – 24 Wood Piles



D Float – 26 Wood Piles



Crossing Between C and D Floats

2 Wood Piles



F Float

12 Wood Piles

E Float 11 Wood Piles



G Float

11 Wood Piles



= Existing Steel Pile





W Float – 8 Steel Piles

2 Additional Steel Piles between W and X



Z Float – 9 Wood Piles

One Additional Wood Pile at bottom of Gangway (furthest Left in above photo)

# WHITTIER SMALL BOAT HARBOR **IMPROVEMENTS - PHASE I**



# **STATE OF ALASKA**

PROJECT

LOCATION

TANKS

200-

AIRSTRI

TLE SHEET & INDEX   1     XISTING CONDITIONS   2     EMOLITION PLAN   3     EW SITE PLAN   4     ST HOLE LOGS   5     REDGING PLAN   6     FESHORE DREDGE DISPOSAL AREA   7	SHEET     BOARDWALK DETAILS (1 OF 3)     BOARDWALK DETAILS (2 OF 3)   14     BOARDWALK DETAILS (2 OF 3)   15     UPLANDS DETAILS   16     GANGWAY PLATFORM DETAILS (1 OF 2)   17     GANGWAY PLATFORM DETAILS (2 OF 2)   18     K FLOAT PLAN   19	INDEX     WATERLINE SECTION & WATER VAULT DETAILS   25     WATER SERVICE DETAILS (1 OF 3)   26     WATER SERVICE DETAILS (2 OF 3)   27     WATER SERVICE DETAILS (3 OF 3)   28     WATER & SEWER SERVICE NOTES   29     GENERAL NOTES (1 OF 4)   30     GENERAL NOTES (2 OF 4)   31	ELECTRICAL P HARBOR ELECTRICAL I DOCK LIGHTING PLAN- FLOAT POWER PLAN- SCHEDULES AND DIAG SPECIFICATIONS AND I DANEL SCHEDULES
VDS OVERVIEW & TYPICAL SECTION   8     NDS PLAN (1 OF 2)   9     NDS PLAN (2 OF 2)   10     ORMASTER BUILDING FOUNDATION PLAN   11     I CELL WALL DETAILS   12	8-FT WIDE MAIN FLOAT UNITS   20     4-FT WIDE FINGER FLOAT UNITS   21     HEADWALK TO CONNECTION FLOAT & PILE HOOP DETAILS-22   21     BULLRAIL & MISCELLANEOUS DETAILS   23     WATER & SEWER SERVICE PLAN   24	GENERAL NOTES (2 OF 4)32 GENERAL NOTES (4 OF 4)33	SCHEDULES AND DIAG







## **DEMOLITION SUMMARY**

## GENERAL

REMOVE THE FOLLOWING ITEMS:

- 1. REMOVE & RELOCATE WEST GANGWAY.
- 2. REMOVE GANGWAY ATTACHMENTS FROM WEST GANGWAY LANDING FLOAT.
- 3. REMOVE & RELOCATE WASTEWATER FLOAT.
- 4. WATER SERVICE UP TO CONSTRUCTION LIMITS.
- SEE ELECTRICAL PLANS FOR ELECTRICAL DEMOLITION 5. REQUIREMENTS.
- 6. FLOAT, PILES, & WALKWAY.
- 7. WEST BOAT GRID & WALKWAY.
- 8. WEST TIMBER APPROACH & ABUTMENT.
- 9. CONCRETE WALKWAYS & AC PAVEMENT AS NOTED.
- 10. RELOCATE BUILDINGS WITHIN PROJECT AREA AS REQ'D.
- 11. OTHER ITEMS AS SHOWN.

LOCATED IN THE UPLANDS PARKING LOT. THE CONTRACTOR SHALL COORDINATE THE STAGING AREA SIZE AND LOCATION WITH THE WHITTIER HARBOR MASTER.





## SOILS CLASSIFICATION, CONSISTENCY AND SYMBOLS

CLASSIFICATION: Identification and classification of the soil is accomplished in general accordance with the ASTM version of the Unified Soil Classification System (USCS) as presented in ASTM Standard D 2487-93. The standard is a qualitative method of classifying soil into the following major divisions (1) coarse grained (2) fine-grained, (3) highly organic soils. Classification is performed on the soils passing the 75 mm (3 inch) sieve and if possible the amount of oversize material (> 75 mm particles) is noted on the soil logs. This is not always possible for drilled test holes because the oversize particles are typically too large to be captured in the sampling equipment. Oversize materials greater than 300 mm (12 inches) are termed boulders, while materials between 75 mm and 300 mm are termed cobbles. Coarse grained soils are those having 50% or more of the non-oversize soil retained on the No. 200 sieve; if a greater percentage of the coarse grains is retained on the No. 4 sieve the coarse grained soil is classified as gravel, otherwise it is classified as sond. Fine grained soils are those having more than 50% of the non-oversize material passing the No. 200 sieve; these may be classified as silt or clay depending their Atterberg liquid and plastic limits or observations of field consistency. Refer to ASTM D 2487-93 for a complete discussion of the classification method.

SOIL CONSISTENCY - CRITERIA: Soil consistency as defined below and determined by normal field and laboratory methods applies only to non-frozen material. For these materials, the influence of such factors as soil structure, i.e. fissure systems, shrinkage cracks, slicken sides, etc., must be taken into consideration in making any correlation with the consistency values listed below. In permafrost zones, the consistency and strength of frozen soils may vary significantly and unexplainably with ice content, thermal regime and soil type.

Relative Density of Sands According to results of Standard Penetration Test			Consistency of Clay in Terms of Unconfined Compressive Strength (tsf)		
Loose Medium Dense Dense Very Dense	N*(bpf) 0 - 10 10 - 30 30 - 60 > 60	Relative Density 0 - 40% 40 - 70% 70 - 90% 90 - 100%	Very Soft Soft Firm Very Firm Hard	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	

\* Standard Penetration, "N": Blows per foot of a 140-pound hammer falling 30 inches on a 1.4" ID split-spoon sampler except where noted.

## SAMPLER TYPE SYMBOLS

Ts .... Shelby Tube Ss .... 1.4" Split Spoon W/ 140# Hammer Cs .... Core Barrel W/ Single Tube Sm.... 2.5" Split Spoon W/ 300# Hammer S<sub>500</sub> .... 2.5" Split Spoon W/ 500# Hammer G .... Grab Sample NOTES:

- 1. SAMPLER TYPES ARE EITHER NOTED ABOVE THE BORING LOG OR ADJACENT TO IT AT THE RESPECTIVE DEPTH.
- 2. SPLIT SPOON SAMPLER SIZES PRESENTED ABOVE REFER TO THE INSIDE DIAMETER OF THE SAMPLER.
- 3. SEE EXISTING CONDITIONS SHEET FOR TEST HOLE LOCATIONS.












•	FOUND MONUMENT	J	GUY ANCHOR
•	FOUND C.O.E. MONUMENT	<b>ڀ</b>	UTILITY POLE
θ	SET REBAR W/ PND YPC	-	STREET SIGN
*	SET NAIL W/ PND SHINER	м	WATER VALVE
8	SANITARY SEWER LIFT STATION	Q	FIRE HYDRANT
0	SANITARY SEWER MANHOLE	+0+	WATER SPIGOT
$\Theta$	SANITARY SEWER CLEANOUT	۵	ANTENNA
O	STORM DRAIN MANHOLE	G	FUEL VAULT
۲	STORM DRAIN CATCH BASIN		PAVED AREA
$\boxtimes$	PHONE/CABLE MANHOLE		CONCRETE WALK
	PHONE/CABLE PEDESTAL	T	BURIED TELEPHONE
6	GAS METER	G	BURIED GAS
0	UTILITY SERVICE CONNECT		BURIED ELECTRIC
	ELECTRIC TRANSFORMER	——————————————————————————————————————	OVERHEAD ELECTRIC
Ε	ELECTRIC MANHOLE	w	WATER LINE
×	LIGHT POLE	s	SANITARY SEWER
	ELECTRIC PEDESTAL	SD	STORM SEWER
۲	ELECTRIC METER	xx	CHAIN LINK FENCE





- THE EXISTING CONDITION
- DOCUMENTATION OF EXISTING BUILDING CONDITION
- THAN 1/4 MILE FROM EXISTING LOCATION
- CONSTRUCTION

- 8. PLACE BUILDING SUPPORTED ON NEW FOUNDATION BEAMS AS
- 9. RECONNECT UTILITIES AND INSTALL PERIMETER FLASHING









305/051020 Whittier Small Boat Harbor/Small Boat Harbor Replacement/Drawings/13 Boardwalk Details 1 of 3.dwg, 13, 11/11/2010 11:294







- 1/2" EXPANSION JOINT, TYPICAL, WITH REMOVABLE PLASTIC JOINT CAP, LIGHT GRAY IN COLOR, WITH

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06/051020 Whittier Small Boat Harbor/Small Boat Harbor Replacement/Drawings/20 8-ft Wide Main Float Units.dwg, 20, 11/11/2010 11:30:

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:2005/051020 Whittier Small Boat Harbor/Small Boat Harbor Replacement/Drawings/27 Water Service Details 2 of 3.dwg, 27, 11/11/2010 11:32:16 AM,

![](_page_92_Figure_0.jpeg)

# WATER & SEWER SYSTEM NOTES

# GENERAL

WATER & SEWER SYSTEM SHALL BE OPERATED DURING SUMMER MONTHS ONLY.

# APPLICABLE CODES

ALL LOCAL CODES PLUS THE FOLLOWING CODES ARE APPLICABLE TO THIS PROJECT: UNIFORM PLUMBING CODE; NFPA 14 (1996 EDITION); AWWA C906 & C901; ASTM SPECIFICATIONS

# SUBMITTALS

THE ENGINEER'S REVIEW OF SUBMITTALS WILL BE FOR GENERAL CONFORMANCE ONLY AND IT SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR TO CONFORM TO ALL REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. ANY INTENDED DEVIATION FROM THE PLANS AND SPECIFICATIONS MUST BE SPECIFICALLY IDENTIFIED BY THE CONTRACTOR AND SPECIFICALLY APPROVED BY THE ENGINEER TO BE ACCEPTABLE.

ALL SUBMITTALS SHALL BE BOUND IN A THREE RING BINDER AS A COMPLETE PACKAGE. SHOP DRAWINGS OF ALL FABRICATED MATERIALS SHALL BE SUBMITTED TO THE ENGINEER FOR WRITTEN APPROVAL PRIOR TO PURCHASE, FABRICATION OR MOBILIZATION OF ANY ITEM. A MINIMUM OF THREE SETS SHALL BE PROVIDED FOR EACH SUBMITTAL, OF WHICH TWO WILL BE RETURNED TO THE CONTRACTOR. THE CONTRACTOR SHOULD ALLOW TWO WEEKS FROM THE TIME OF RECEIPT FOR REVIEW OF SUBMITTALS BY THE ENGINEER FOR A REASONABLE NUMBER OF DRAWINGS.

CERTIFICATIONS, MANUFACTURER'S DATA AND OTHER INFORMATION FOR ALL MATERIALS, INCLUDING THOSE NOT SPECIFICALLY SHOWN IN THESE NOTES OR ON INDIVIDUAL DRAWINGS, SHALL BE SUBMITTED TO THE ENGINEER FOR WRITTEN APPROVAL TO VERIFY CONFORMANCE WITH THE PLANS AND SPECIFICATIONS. IN THE EVENT THAT THE PLANS AND SPECIFICATIONS DO NOT SPECIFICALLY REFERENCE A MATERIAL, THE APPROVAL OF MATERIALS WILL BE BASED ON ITS CONFORMANCE TO THE ABOVE REFERENCED APPLICABLE CODES. ALL METHODS AND MATERIALS SHALL CONFORM TO THESE GENERAL NOTES, GOOD WORKMANSHIP, GENERALLY ACCEPTED INDUSTRY STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.

# MATERIALS AND CONSTRUCTION

# PIPE AND FITTINGS

ALL WATER SYSTEMS PIPE AND PIPE FITTINGS SHALL BE MADE FROM HIGH DENSITY POLYETHYLENE (HDPE), AND APPROVED BY THE NATIONAL SANITATION FOUNDATION (NSF). 3-INCH POTABLE WATER LINES SHALL BE SDR 11 HDPE RATED FOR 160 PSI; 4-INCH AND 6-INCH POTABLE WATER LINES SHALL BE SDR 13.5 HDPE RATED FOR 130 PSI; 6-INCH FIRE LINES SHALL BE SDR 9 RATED FOR 200 PSI UNLESS NOTED OTHERWISE

ALL SEWER SYSTEM PIPE AND FITTINGS SHALL BE MADE FROM HDPE AND APPROVED BY THE NSF. 4-INCH SEWER FORCE MAIN SHALL BE SDR 13.5 RATED FOR 130 PSI.

ALL HDPE PIPE SHALL BE INSTALLED IN A MANNER SUFFICIENT TO ACCOMMODATE FOR THERMAL EXPANSION AND CONTRACTION. THE PIPE SHALL BE FUSED BY AN INDIVIDUAL WHO HAS BEEN CERTIFIED BY THE PIPE SUPPLIER OR FUSION MACHINE MANUFACTURER.

FUSION WELDED PIPES SHALL BE DONE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. FUSED JOINTS SHALL EXHIBIT SMALL, SMOOTH, UNIFORM, WELL-ROUNDED JOINT BEADS. JOINTS WITH SHARP EDGED, IRREGULAR OR PARTICULARLY LARGE BEADS WILL BE REJECTED AND REPLACED. THE NUMBER OF PIPE JOINTS, WELDED OR FLANGED SHALL BE KEPT TO A MINIMUM, WITH NO MORE THAN AN AVERAGE OF ONE EVERY 20 FEET

FITTINGS SHALL MEET THE SAME PRESSURE AND INTEGRITY STANDARDS AS THE PIPE.

# PIPE HANGERS AND CLAMPS

PIPE HANGER SPACING SHALL BE EVERY 8 FEET MAX. UNLESS NOTED OTHERWISE AND AT THE END OF FACH FLOAT

## HARDWARE AND BOLTS

ALL MISCELLANEOUS STEEL PLATES AND SHAPES SHALL BE ASTM A36 GALVANIZED.

ALL BOLTS, LAG SCREWS, NUTS AND WASHERS SHALL BE GALVANIZED STEEL, HEAVY SEMI-FINISHED HEX NUTS, UNLESS OTHERWISE NOTED. WASHERS SHALL BE USED UNDER BOTH HEAD AND NUT OF ALL BOLTS.

ALL GALVANIZED BOLTS, LAG SCREWS, NUTS, WASHERS AND NAILS SHALL MEET ASTM A307 OR SIMILAR REQUIREMENTS AS APPROVED BY THE ENGINEER AND SHALL BE HOT-DIPPED GALVANIZED. MALLEABLE IRON WASHERS SHALL BE USED IN ALL AREAS WHERE THE BOLT HEAD OR NUT BEAR AGAINST WOOD, EXCEPT UNDER ECONOMY HEADS

### WATER SYSTEM COMPONENTS

THE BACKFLOW PREVENTER SHALL BE A WATTS MODEL NUMBER 909 RPDA SERIES REDUCED-PRESSURE-ZONE BACKFLOW PREVENTER OR APPROVED EQUAL.

THE WATER SERVICE VAULT SHALL BE A MODIFIED UTILITY VAULT MODEL #687-LA, OR APPROVED EQUAL, WITH AN EXTERIOR WATERPROOF MEMBRANE AND INTERIOR ACCESS LADDER. A CLEAR DISTANCE OF 10 FEET MINIMUM SHALL BE MAINTAINED BETWEEN WALLS TO ACCOMMODATE WATER UTILITIES. MINIMUM WEIGHT OF VAULT SHALL BE 40,000 POUNDS. THE MANHOLE SHALL BE RATED FOR AASHTO H-20 LOADING, AND STAMPED "WATER" IN 3" LETTERING. A CLEARANCE OF 12 INCHES SHALL BE MAINTAINED BETWEEN THE FLOOR AND WALLS OF THE VAULT AND THE EQUIPMENT INSIDE.

THE BALL VALVE SHALL BE PVC. AND RATED FOR 150 PSI. ASAHL OR APPROVED EQUAL.

THE 3" CHECK VALVE SHALL BE A RED VALVE TIDE-FLEX SERIES TF-1 CHECK VALVE, OR APPROVED FOUAL

THE 1" HDPE FLEX HOSE SHALL BE PERFORMANCE PIPE SIDR 7 (IPS) POTABLE WATER TUBING. ALL FITTINGS SHALL BE BRASS WATER WORKS FLARE FITTINGS, COMPATIBLE WITH THE TUBING MATERIAL.

FLEX HOSE SHALL BE 4-INCH TITAN EW328 POTABLE WATER HOSE OR APPROVED EQUAL, PROVIDE PREMANUFACTURED GALVANIZED FLANGE CONNECTION AT EACH END.

THE HOSE BIBS SHALL BE INSTALLED WITH A VACUUM BREAKER, WATTS NO. 8A SERIES OR APPROVED EQUAL

GATE VALVE NEAR HYDRANT SHALL BE RESILIENT SEAT GATE VALVE RATED AT 250 PSI CONFORMING TO AWWA C-509 "U.S. PIPE METROSEAL 250" OR EQUAL.

FIRE HYDRANT SHALL HAVE A CAST-IRON BODY, COMPRESSION TYPE VALVE OPENING AGAINST PRESSURE AND CLOSING WITH PRESSURE, NPS 6 MECHANICAL-JOINT INLET, 150 PSIG WORKING PRESSURE AND CONFORM TO NFPA 1963, WITH EXTERNAL HOSE THREAD USED BY LOCAL FIRE DEPARTMENT. INCLUDE CAST IRON CAPS WITH STEEL CHAINS. HYDRANT GUARD BOLLARDS SHALL BE INSTALLED AT EACH HYDRANT I OCATION

WATER METER INSIDE WATER SERVICE VAULT SHALL BE A 4" POSITIVE DISPLACEMENT TYPE, INVENSYS OR APPROVED EQUAL.

## SEWER SYSTEM COMPONENTS

FLEX HOSE SHALL BE 4-INCH TITAN EW328 POTABLE WATER HOSE OR APPROVED EQUAL. PROVIDE PREMANUFACTURED GALVANIZED FLANGE CONNECTION AT EACH END.

# FIRE SYSTEM COMPONENTS

FIRE EXTINGUISHER CABINETS SHALL BE A CHEYENNE MODEL FEH-L OR APPROVED EQUAL, MOUNTED TO THE FIRE EXTINGUISHER CABINET STAND AS REQUIRED. CABINETS SHALL INCLUDE FIRE EXTINGUISHERS WITH A MINIMUM RATING OF 2A, 20-B.C. CABINETS SHALL HAVE A SIGN CLEARLY IDENTIFYING IT AS CONTAINING A FIRE EXTINGUISHER.

FIRE DEPARTMENT INLET CONNECTIONS SHALL BE A 4" STORZ TYPE AND RATED FOR 1250 GPM. THE 4-INCH DIAMETER FLEX HOSE SHALL BE GOODYEAR PLICORD BREWLINE, OR APPROVED EQUAL, WITH COMPATIBLE FITTINGS. THE HOSE AND FITTINGS SHALL BE RATED FOR 200 PSI. ANGLE VALVES SHALL BE MANUFACTURED BY POWHATAN, OR APPROVED EQUAL.

## LIFE RING CABINETS

LIFE RING CABINETS SHALL BE A CHEYENNE MODEL LRHC-2, OR APPROVED EQUAL. CABINETS SHALL BE MOUNTED TO THE LIFE RING CABINET STAND AS REQUIRED. CABINETS SHALL HAVE A SIGN CLEARLY IDENTIFYING IT AS CONTAINING A LIFE RING.

### FLOAT SIGNAGE

SIGNS SHALL BE MOUNTED ONTO THE TIMBER WALER PORTION OF THE FLOAT DECK ABOVE AND ADJACENT TO THE VALVE. SIGNS SHALL BE INSTALLED WITH SLOTTED HOLES FOR SHRINKAGE AND EXPANSION. SUBMIT DETAILS FOR APPROVAL. PROVIDE ALL SHUT-OFF, AND DRAIN VALVES WITH SIGNS IDENTIFYING THE TYPE OF VALVE AND THE AREA AFFECTED BY THE VALVE. SIGNS SHALL BE 1/8" THICK ENGRAVING STOCK MELAMINE PLASTIC LAMINATE, COMPLY WITH FSL-P-387. HELVETICA LETTER STYLE OF MINIMUM 1/4-INCH HIGH RED LETTERS ON A WHITE BACKGROUND

TESTING AND DISINFECTION PRESSURE TESTING, FLUSHING, AND DISINFECTION SHALL BE CONDUCTED IN ACCORDANCE WITH THE UNIFORM PLUMBING CODE, MANUFACTURERS RECOMMENDATIONS, AND CITY OF WHITTIER STANDARD SPECIFICATIONS. THE CONTRACTOR SHALL CONDUCT TESTING AND DISINFECTION UNDER THE SUPERVISION OF THE ENGINEER PRIOR TO FINAL CONNECTION TO THE EXISTING WATER MAIN. THE CONTRACTOR SHALL PROVIDE PLUGS AND TEMPORARY BLOW-OFF ASSEMBLIES FOR PRESSURE TESTING ALL LINES AND DISINFECTING THE POTABLE WATER LINES.

THE POTABLE WATER SHALL BE HYDROSTATICALLY TESTED TO 120 PSI (1.5 TIMES THE SYSTEM DESIGN OPERATING PRESSURE). INITIALLY, THE TEST PRESSURE SHALL BE APPLIED TO THE HDPE PIPE FOR 3 TO 4 HOURS TO ALLOW FOR THE INITIAL EXPANSION OF THE PIPE. AFTER THIS PERIOD, ADD ADDITIONAL MAKE-UP WATER AND RETURN TO THE FINAL TEST PRESSURE FOR A MINIMUM OF 30 MINUTES. DURING THE 30 MINUTE TESTING PERIOD, WATER PRESSURE MAY DROP BY NO MORE THAN 5 PSI, DISINFECTING THE POTABLE WATER SYSTEM SHALL BE IN ACCORDANCE WITH CITY SPECIFICATIONS. THE POTABLE WATER SYSTEM SHALL PASS THE PRESSURE TEST AND DISINFECTING REQUIREMENTS AND BE FLUSHED PRIOR TO CONNECTING TO THE CITY WATER SYSTEM. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY VALVES AND CAPS TO PERFORM THE TEST.

AS-BUILT PLANS COMPLETION OF THE WATER SYSTEM.

WATER SERVICE SUBMITTAL LIST THE FOLLOWING IS A PARTIAL LIST OF SUBMITTALS REQUIRED FOR THE WATER SERVICE PORTION OF THIS PROJECT. THIS DOES NOT CONSTITUTE A COMPLETE LIST AS IT WILL DEPEND UPON THE CONTRACTOR'S METHODS

CONSTRU	UCTION PI	LANS,
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## SHOP DRAWINGS AND MATERIAL CERTIFICATIONS

-	
	HDPE PIPE AND
	WATER VAULT
	PIPE SUPPORT I
	ALL U-BOLT, BO
	POTABLE WATER
	HOSE WEIGHTS
	CHECK VALVE
	PIPE SUPPORTS
	PROTECTIVE BOL
0.	GATE VALVES A
1.	WATER METER
2.	BACK FLOW PRE

- 13. HOSE BIBS
- 14. HDPE FLEX HOSE AND FITTINGS
- 15. AIR HOSE CONNECTION 16. PRESSURE TESTING RESULTS
- 17. RED-LINED AS-BUILT PLANS

WHITTIER ALLIST	CITY OF WHITTIER PO BOX 608		PND Engineers, Inc. is not responsible for safety programs, methods or procedures of operation, or the construction of the design shown on these drawings. Where specifications are general or not called out, the specifications shall conform to standards of industry. Drawings are for use on this project only and are not	1506 West 36th Avenue Anchorage, Alaska 99503	WHITTIER SMALL BOAT HARBOR IMPROVEMENTS - PHASE I
	WHITTIER, ALASKA 99693	<b>OPEN CELL®</b> AND <b>OPEN CELL SHEET PILE®</b> ARE REGISTERED TRADEMARKS OF PND ENGINEERS, INC.	intended for reuse without written approval from PND. Drawings are also not to be used in any manner that would constitute a detriment directly or indirectly to PND.	Phone: 907.561.1011	WATER & SEWER SERVICE NOTES
on July 5.	PHONE: (907) 472-2327 FAX: (907) 472-2404	PATENT – US 7,018,141 B2 PATENT – US 7,488,140 B2	REV DATE DESCRIPTION	www.pndengineers.com	DESIGNED BY: MIB DATE: NOVEMBER 2010 CHECKED BY: DST PROJECT NO: 051020 0F 33

![](_page_93_Picture_55.jpeg)

THE CONTRACTOR SHALL SUBMIT THREE SETS OF COMPLETED AS-BUILT PLANS TO THE ENGINEER UPON

(INCLUDES DRAWINGS AND WRITTEN DESCRIPTION OF METHODS) PEDESTRIAN CONTROL PLAN FOR CONNECTION TO EXISTING UTILITY MAIN. ER VAULT AND WATER LINE INSTALLATION PLAN.

FITTINGS OLTS AND MISCELLANEOUS HARDWARE HOSE

LARDS AND BALL VALVES

EVENTER

![](_page_93_Picture_63.jpeg)

# **GENERAL NOTES**

OWNER: CITY OF WHITTIER

THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL BE POSTED PROMINENTLY AT THE CONTRACTOR'S ONSITE PROJECT OFFICE.

ANY DISCREPANCIES FOUND AMONG THE DRAWINGS. SPECIFICATIONS, SITE CONDITIONS, AND THESE NOTES. SHALL BE REPORTED TO THE OWNER/ENGINEER AT ONCE. ANY FURTHER WORK PERFORMED BY THE CONTRACTOR AFTER FINDING SUCH DISCREPANCIES SHALL BE DONE AT HIS OWN RISK.

OPEN CELL / PATENT NOS. 6,715,964.B2 AND 7,018,141.B2 AND 7,488,140.B2

# APPLICABLE CODES

ALL LOCAL CODES PLUS THE FOLLOWING SPECIFICATIONS, STANDARDS AND CODES ARE PART OF THESE GENERAL NOTES:

- INTERNATIONAL BUILDING CODE, 2010 EDITION AWS D1.1 STRUCTURAL WELDING CODE 2010 EDITION 2.
- ASTM SPECIFICATIONS ASCE 7 - 2005
- IN THE EVENT THAT THERE IS A CONFLICT BETWEEN THE ABOVE REFERENCES AND THESE GENERAL NOTES
- THE FOLLOWING PRIORITY WILL BE FOLLOWED:
  - ALL PROJECT PERMIT REQUIREMENTS THESE GENERAL NOTES AND PLANS
  - LOCAL CODES (& MOA SPECIFICATIONS) 3
  - THE SPECIFICATIONS, STANDARDS AND CODES LISTED ABOVE IN ORDER OF PRECEDENCE

### DESIGN CRITERIA FOR NEW FLOATS

DEAD LOADS - ALL FLOAT LIVE LOAD - UNIFORM 40 PSF, 1000 POUND CONCENTRATED FLOAT SNOW LOAD - UNIFORM 40 PSF WIND LOAD - 120 MPH 3-SECOND WIND SPEED, EXPOSURE D REDUCED TO 1-MINUTE WIND WITH VESSEL SHAPE FACTORS VESSEL SIZE - MAXIMUM LENGTH EQUAL TO ADJACENT FINGER FLOAT LENGTH + X FT.

TIDAL LEVELS - ELEVATION DATUM FOR THIS PROJECT IS 0.0 MEAN LOWER LOW WATER

EXTREME HIGH WATER	+18.7 FT.
HIGH TIDE LINE (HTL)	+15.5 FT.
MEAN HIGHER HIGH WATER (MHHW)	+12.3 FT.
MEAN HIGH WATER (MHW)	+11.3. FT.
MEAN TIDE LINE (MTL)	+6.4 FT.
MEAN LOW WATER (MLW)	+1.5 FT.
MEAN LOWER LOW WATER (MLLW)	+0.0 FT.
EXTREME LOW WATER	-5.0 FT.

## SURVEY DATUM

HORIZONTAL CONTROL IS ALASKA STATE PLANE ZONE 4 NAD 83 METERS CONVERTED TO FEET. CONTROL IS BASED ON THE PUBLISHED 1992 VALUE OF THE USC&GS CONTROL MONUMENT "MARGIN 1956", WHICH HAS SINCE BEEN DESTROYED. TO INSURE THE CORRECT POSITION OF POINTS LOCATED BY "GPS", THE CONTRACTOR MUST PERFORM A "GPS SIGHT CALIBRATION" PRIOR TO THE USE OF GPS FOR LAYOUT PURPOSES.

# SURVEY

ALL CONSTRUCTION SURVEYS SHALL BE PERFORMED BY OR UNDER THE SUPERVISION OF A SURVEYOR LICENSED IN THE STATE OF ALASKA

AN ACCURATE METHOD OF HORIZONTAL CONTROL SHALL BE ESTABLISHED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER BEFORE CONSTRUCTION BEGINS. THE CONTRACTOR SHALL MAINTAIN THE CONTROL SYSTEM THROUGHOUT THE PROJECT. IF AT ANY TIME THE METHODS UTILIZED FAIL TO PROVIDE ACCURATE LOCATION THE CONTRACTOR MAY BE REQUIRED TO SUSPEND WORK. THE CONTRACTOR SHALL LAY OUT THE WORK FROM OWNER ESTABLISHED VERTICAL CONTROL POINTS AND CONTRACTOR ESTABLISHED HORIZONTAL CONTROL POINTS AND SHALL BE RESPONSIBLE FOR ALL REQUIRED MEASUREMENTS TAKEN FROM THESE POINTS.

THE CONTRACTOR SHALL FURNISH AT ITS OWN EXPENSE ALL STAKES, TEMPLATES, PLATFORMS, EQUIPMENT, RANGE MARKERS, AND LABOR AS MAY BE REQUIRED TO LAY OUT THE WORK FROM THE CONTROL POINTS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN THE CONTROL POINTS UNTIL AUTHORIZED TO REMOVE THEM. IF SUCH POINTS ARE DESTROYED OR DISTURBED THEY SHALL BE REESTABLISHED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.

#### STAGING AREA

THE UPLAND STAGING AREA NOTED ON THE PLANS WILL BE AVAILABLE FOR USE BY THE CONTRACTOR FOR THE DURATION OF THE CONTRACT. THE CONTRACTOR SHALL NOT BLOCK THE ACCESS TO ADJOINING PROPERTIES OR TRAFFIC. ANY DAMAGE CAUSED BY THE CONTRACTOR OR HIS SUB CONTRACTORS TO THE PROPERTY OR ADJOINING PROPERTY SHALL BE REPAIRED IMMEDIATELY AT THE EXPENSE OF THE CONTRACTOR. THE CONTRACTOR WILL BE SOLELY RESPONSIBLE FOR USE OF THE SITE. THE CONTRACTOR IS REQUIRED TO CLEAN THE SITE STAGING AREA AS PART OF THE FINAL CLEANUP WORK ON THE PROJECT.

# MATERIALS AND CONSTRUCTION

#### GENERAL

THE FOLLOWING SECTION COVERS ALL PHASES OF CONSTRUCTION FOR THIS PROJECT, EXCEPT SEWER WATER AND ELECTRICAL REQUIREMENTS WHICH ARE COVERED ELSEWHERE. ADDITIONAL SPECIFIC REQUIREMENTS ARE COVERED IN THE PLANS. REQUIREMENTS COVERED ON THE PLANS SUPERSEDE THOSE IN THIS SECTION IN CASE OF CONFLICT.

MATERIALS NOT SPECIFICALLY NOTED IN THESE GENERAL NOTES OR ELSEWHERE ON THE DRAWINGS SHALL BE SUBMITTED BY THE SUPPLIER FOR APPROVAL. APPROVAL WILL BE BASED ON CONFORMANCE TO CURRENT STANDARDS UTILIZED BY THE OWNER. ALL MATERIALS MUST CONFORM TO GOOD WORKMANSHIP, ACCEPTABLE INDUSTRY STANDARDS AND MANUFACTURERS RECOMMENDATIONS.

CONSTRUCTION NOT MENTIONED IN THESE GENERAL NOTES SHALL BE PERFORMED USING REASONABLE CARE AND GOOD CONSTRUCTION PRACTICES. FINAL INSPECTION AND ACCEPTANCE OF ALL WORK NOT SPECIFICALLY INCLUDED IN THESE GENERAL NOTES OR ON THE DRAWINGS SHALL BE MADE BY THE ENGINEER. APPROVAL OF ALL METHODS AND PRODUCTS SHALL BE BASED UPON CONFORMANCE TO THE GENERAL NOTES. DRAWINGS, QUALITY OF WORKMANSHIP, APPLICABLE INDUSTRY STANDARDS, AND PERTINENT MANUFACTURERS RECOMMENDATIONS

### FLOATS - GENERAL

THE FLOAT MANUFACTURER SHALL HAVE A MINIMUM OF FIVE (5) YEARS EXPERIENCE IN THE DESIGN AND MANUFACTURE OF TIMBER FLOATS. THE TIMBER FLOAT MANUFACTURING FACILITY SHALL PROVIDE THE PROPER ENVIRONMENT AND PHYSICAL CONDITIONS NECESSARY FOR CONSTRUCTION OF HIGH QUALITY TIMBER FLOAT UNITS

WALKING SURFACES OF FLOAT UNITS SHALL BE LEVEL AND FLUSH WITH ADJOINING FLOAT UNITS AND SHALL FLOAT LEVEL UNDER DEAD LOAD. THE MAXIMUM SLOPE OF THE FLOAT DECK UNDER DEAD LOAD IS ONE (1) INCH PER TEN (10) FEET OF FLOAT LENGTH OR WIDTH. THE MAXIMUM HEIGHT VARIATION BETWEEN ADJOINING SURFACES SHALL BE 1/4-INCH.

AS EACH FLOAT UNIT IS UNIQUE TO A SPECIFIC LOCATION, IDENTIFICATION OF FLOAT UNITS IS CRUCIAL. ALL FLOATS SHALL BE CLEARLY IDENTIFIED WITH PLACARDS SECURELY ATTACHED ON ONE SIDE AND ONE END. PLACARD INFORMATION SHALL INCLUDE THE DATE OF MANUFACTURE, SPECIFIC FLOAT TYPE, JOB NUMBER, AND INDIVIDUAL FLOAT DESIGNATION LOCATED IN THE FLOAT SCHEDULE. IN ADDITION, MAIN FLOAT SCHEDULE DESIGNATIONS SHALL BE PAINTED WITH 4" WHITE STENCILED LETTERS ON THE OUTSIDE CENTER OF ONE BULLRAIL. FLOAT IDENTIFICATION LOCATIONS SHALL BE CONSISTENT AND CLEARLY VISIBLE

THE FLOATS SHALL BE LIFTED AND SUPPORTED DURING STOCKPILING, TRANSPORTING, AND ASSEMBLY ONLY AT LIFTING OR SUPPORTING POINTS AS SHOWN ON THE APPROVED SHOP DRAWINGS. FLOATS SHALL BE PROTECTED AGAINST DAMAGE FROM ANY CAUSE. ANY DAMAGED UNITS SHALL BE REJECTED OR REPAIRED AT THE ENGINEERS OPTION. REPAIR OR REPLACEMENT SHALL BE DONE AT NO ADDITIONAL COST TO THE OWNER.

THE FIRST TWO FLOAT UNITS PRODUCED OF EACH WIDTH AND LENGTH SHALL BE CHECKED FOR FREEBOARD PRIOR TO ASSEMBLY OF REMAINING FLOATS OF THAT WIDTH AND LENGTH. FLOATS SHALL BE PLACED IN SALT WATER AND ERFEBOARD MEASURED.

IF ADJUSTMENTS TO FREEBOARD ARE REQUIRED AS DETERMINED BY THE ENGINEER THE FOAM BILLET CONFIGURATION WILL BE MODIFIED. COST TO MODIFY FOAM BILLETS WILL BE NEGOTIATED WITH THE OWNER AT THAT TIME.

#### POLYSTYRENE

FLOAT INNER CORES SHALL BE CLOSED-CELL EXPANDED POLYSTYRENE IN ACCORDANCE WITH ASTM C578. THE DENSITY OF THE POLYSTYRENE SHALL BE BETWEEN 0.9 AND 1.0 POUNDS PER CUBIC FOOT AND BILLETS SHALL NOT CONTAIN GREATER THAN 5% REGRIND MATERIAL. THE FOAM SHALL HAVE A MAXIMUM ADSORPTION OF 4 PERCENT BY VOLUME AS TESTED BY ASTM C-272. FOAM SHALL HAVE A 10 PSI MINIMUM COMPRESSIVE STRENGTH AT 10% DEFORMATION AND A 25 PSI MINIMUM FLEXURAL STRENGTH. BILLETS SHALL HAVE A VARIATION IN DESIGN DIMENSION OF LESS THAN 1/8 INCH.

FOAM CORE SHALL BE MADE OF NOT MORE THAN 4 LAMINATED SECTIONS. LAMINATIONS SHALL BE GLUED WITH A LOW SOLVENT GLUE, AND SHALL BE STRAPPED TO PREVENT DELAMINATION DURING TRANSPORTATION AND HANDLING. LAMINATIONS SHALL BE PLACED IN SUCH A MANNER SO AS NOT TO PUT UNDO STRAIN FROM BENDING OR OTHER LOADING ON LAMINATED JOINTS.

POLYSTYRENE FOAM BILLETS SHALL BE COATED ON ALL SIDES WITH A 65-MIL DFT OF 100% SOLID POLYURETHANE. ALL UTILITY TRENCHES SHALL BE COATED ON ALL SIDES TO A THICKNESS OF 75-MIL DFT.

VERTICAL SEAMS AS SHOWN ON THE PLANS ARE ALLOWED IF ALL EXTERNAL SURFACES OF EACH INDIVIDUAL BILLET ARE COATED WITH 50-MIL DFT OF POLYURETHANE. BILLET DIMENSIONAL TOLERANCES SHALL BE AS FOLLOWS: (SUBMIT BILLET GEOMETRY FOR ENGINEER REVIEW.)

WIDTH- MAXIMUM OF 1/4" GAP BETWEEN BILLET AND ADJACENT GLULAM. LENGTH- MAXIMUM OF 1/2" GAP BETWEEN BILLET AND ADJACENT DIAPHRAGM. EXTRA POLYSTYRENE FOAM BILLETS 35 FULL-SIZE (10"x20"x9') AND 25 HALF-SIZE (10"x20"x4'-6") POLYSTYRENE LEVELING BILLETS SHALL BE PROVIDED IN THE BASE CONTRACT AND CONSIDERED INCIDENTAL TO THE FLOAT SYSTEM THESE BILLETS SHALL BE COATED PER THE POLYSTYRENE SPECIFICATION FOUND IN THESE GENERAL NOTES.

THE CONTRACTOR SHALL INSTALL EXTRA LEVELING BILLETS UNDER FLOATS AS REQUIRED TO OBTAIN THE FOLLOWING FREEBOARD REQUIREMENTS AS MEASURED BY THE ENGINEER. FINAL FREEBOARD AND SLOPES SHALL BE MEASURED AFTER ALL FLOAT UTILITIES AND ATTACHMENTS HAVE BEEN INSTALLED.

OF FLOAT UNITS

# FLOAT INSTALLATION

STRUCTURAL STEEL PILES STEEL PILES FOR FLOATS SHALL CONFORM TO ASTM A252, GRADE 3 WITH ASTM A36 CHEMISTRY SUITABLE FOR WELDING OR APPROVED EQUIVALENT. ALL PILES SHALL BE GALVANIZED. SPIRAL WELD PILES ARE NOT ALLOWED

FLOAT PILE INSTALLATION ALL FLOAT PILES SHALL BE DRIVEN PLUMB THROUGH THE PILE HOOPS IN SUCH A MANNER TO ALLOW THE FLOAT FULL MOVEMENT THROUGH THE TIDES WITHOUT BINDING. EACH FLOAT PILE SHALL BE CAPABLE OF 10,000 POUNDS OF BEARING. SHOULD VIBRATORY HAMMER REFUSAL OCCUR PRIOR TO ACHIEVING COMPLETE EMBEDMENT, SEE "K FLOAT PLAN" SHEET FOR FLOAT PILE & GANGWAY PILE BEARING REQUIREMENTS.

PIPE PILE DRIVING ALL PILES SHALL BE DRIVEN. THE CONTRACTOR SHALL SUBMIT A PLAN FOR PILE DRIVING. THE PLAN SHALL CONTAIN HAMMER TYPE AND DRIVING METHOD FOR ALL PIPE TYPES. THE CONTRACTOR SHALL NOT MOBILIZE HAMMERS AND RELATED FOUIPMENT PRIOR TO RECEIVING WRITTEN APPROVAL OF THE PLAN. THE CONTRACTOR SHOULD ALLOW ONE WEEK FOR REVIEW OF THE PLAN BY THE ENGINEER. ALL PILE DRIVING METHODS SHALL MEET THE REQUIREMENTS OF THE PERMITS ISSUED FOR THIS PROJECT.

ALL FLOAT PILES SHALL BE DRIVEN WITH A VIBRATORY HAMMER, APE 200 OR EQUIVALENT AND IMPACT HAMMER AS REQUIRED TO THE NECESSARY EMBEDMENT AS SHOWN ON THE DRAWINGS, ALL GANGWAY PILES SHALL BE DRIVEN WITH AN IMPACT HAMMER HAVING A MINIMUM 45,000 FT-LBS ENERGY TO OBTAIN NECESSARY EMBEDMENT & CAPACITY AS SHOWN ON THE DRAWINGS. PILE CAPACITY AND EMBEDMENT SHALL BE DETERMINED SOLELY BY THE ENGINEER.

ANY HAMMER THAT CAUSES DAMAGE TO THE PILES DURING DRIVING OPERATIONS SHALL BE SUBSTITUTED. WITH AN ACCEPTABLE ALTERNATE HAMMER AT NO ADDITIONAL EXPENSE TO THE OWNER. IMPACT HAMMERS SHALL BE SUPPLIED WITH NEW CAPBLOCK CUSHIONS, WHICH SHALL BE CHANGED AT THE MANUFACTURER'S RECOMMENDED CYCLE. THE CONTRACTOR'S DRIVING PLAN SHALL INCLUDE MANUFACTURER'S RECOMMENDATIONS AND INFORMATION ON HAMMER CUSHIONS.

PILES SHALL BE PLACED WITHIN 1% OF SPECIFIED VERTICAL ALIGNMENT AND WITHIN 2 INCHES OF SPECIFIED LOCATION AT CUTOFF. PILES HITTING OBSTACLES, MISALIGNED PILES AND PILES THAT HAVE NOT ACHIEVED MINIMUM PENETRATION PRIOR TO REFUSAL SHALL BE PULLED BY THE CONTRACTOR WITH A VIBRATORY HAMMER AND REDRIVEN AT NO ADDITIONAL COST TO THE OWNER. A VIBRATORY HAMMER WITH A MINIMUM OF 300 HORSEPOWER AND MINIMUM ECCENTRIC MOMENT OF 4.000 IN-LBS OR AS OTHERWISE APPROVED BY ENGINEER MUST BE AVAILABLE AND ON SITE DURING ALL PIPE PILE DRIVING OPERATIONS.

PILE LENGTHS SHALL BE SUPPLIED AS SPECIFIED.

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NEW TIMBER FLOATS: AS DETAILED IN THE PLANS MINIMUM w/ A MAX DIFFERENCE OF 1" BETWEEN CORNERS

THE CONTRACTOR SHALL SUBMIT A BILLET INSTALLATION PLAN FOR ENGINEER APPROVAL. BILLET INSTALLATION SHALL BE PROVIDED IN THE BASE CONTRACT & CONSIDERED INCIDENTAL TO FLOAT INSTALLATION.

ALL NEW AND EXISTING FLOATS SHALL BE HANDLED IN A MANNER THAT WILL NOT CAUSE DAMAGE TO THE FLOAT. ALL FLOAT CONNECTIONS ARE DESIGNED TO ACCOMMODATE THE REQUIRED VESSELS LOADS ONLY WHEN INSTALLED AS A COMPLETE FLOAT SYSTEM AS SHOWN IN THESE PLANS. DAMAGE TO THE FLOAT CONNECTION HARDWARE AND FLOAT STRUCTURE WILL OCCUR IF FLOATS ARE LEFT IN PLACE WITHOUT THE PROPER SUPPORT STRUCTURES AROUND THEM. THE CONTRACTOR SHALL REPAIR AND/OR REPLACE AT THE OWNER'S PREFERENCE, AND AT NO COST TO THE OWNER, ANY FLOAT AND/OR STRUCTURAL COMPONENT THAT IS DAMAGED BECAUSE OF IMPROPER INSTALLATION OR SUPPORT.

ALL PILE INSTALLATIONS SHALL BE CONDUCTED WITH ENGINEER PRESENT. THE CONTRACTOR SHALL ASSIST THE ENGINEER IN MONITORING THE PILE DRIVING. THE CONTRACTOR SHALL MARK EACH PILE WITH ONE-FOOT INCREMENTS WITH EVERY FIVE-FOOT INCREMENT NUMBERED. FOR DETERMINATIONS OF PILE REFUSAL OR CAPACITY, THE CONTRACTOR SHALL MARK THE PILES WITH ONE-INCH INCREMENTS DURING THE FINAL DRIVE. THE MARKS SHALL BE VISABLE/READABLE FROM ALL SIDES OF THE PILE.

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# WHITTIER SMALL BOAT HARBOR **IMPROVEMENTS - PHASE I**

**GENERAL NOTES** (1 OF 4)

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# **GENERAL NOTES** (CONT.)

ALL RECLAIMED STEEL PILES, EXTRA NEW STEEL PILES AND STEEL PIPE PILE CUTOFFS 10' AND LONGER SHALL BE DELIVERED TO THE OWNER, CUTOFFS SHORTER THAN 10' SHALL BECOME THE PROPERTY OF THE CONTRACTOR. THE CONTRACTOR SHALL RETRIEVE PIPE CUTOFFS THAT FALL INTO HARBOR WATER. THE CONTRACTOR SHALL REMOVE THE PIPE CUTOFFS FROM THE PROJECT SITE.

PILES SHALL BE DRIVEN TO REQUIRED PILE CAPACITY AND EMBEDMENT AS SHOWN ON THE DRAWINGS. PILE CAPACITY AND EMBEDMENT WILL BE DETERMINED SOLELY BY THE ENGINEER

PILE SPLICES SHALL BE COMPLETE JOINT PENETRATION WITH BACKING RINGS PER AWS SPECIFICATIONS. CARE SHALL BE TAKEN THAT MEMBERS REMAIN IN STRAIGHT ALIGNMENT THROUGH SPLICES. NO PIECE OF PILE LESS THAN 5 FEET LONG SHALL BE SPLICED.

# FLAT SHEET PILE MATERIALS

ALL FLAT SHEET PILES SHALL BE NEW PS31 AND PS27.5 AS SHOWN ON THE DRAWINGS. ALL PS31 SHEET PILE, WYE PILES AND X-PILES SHALL BE HOT-DIPPED GALVANIZED PER ASTM A123 AND ASTM A385, ALL OTHER FLAT SHEET PILES SHALL BE UNCOATED. ALL FLAT SHEET PILE SHALL MEET THE REQUIREMENTS OF ASTM A328 OR A572 GRADE 50 WITH A MAXIMUM CARBON EQUIVALENCY (CE) OF 0.44, CALCULATED PER THE LATEST EDITION OF AWS D1.1, AND BE INSTALLED FULL LENGTH WITHOUT SPLICES. MINIMUM INTERLOCK TENSILE STRENGTH SHALL BE 20,000 LBS PER INCH, AND ALLOWABLE FLAT SHEET INTERLOCK SWING ANGLE SHALL BE 7 MINIMUM FROM CENTER, EACH WAY. SHEET PILES SHALL BE PROVIDED WITH 2.5-INCH DIAMETER PICKING EYES, WITH A CENTER OF HOLE TO EDGE DISTANCE OF 6 INCHES, PICKING HOLES SHALL BE PROVIDED ON AT LEAST ONE END OF ALL SHEET PILES AND FABRICATED ITEMS.

A PORTION OF THE REQUIRED SHEET PILE IS PROVIDED BY THE OWNER. A DETAILED OWNER SUPPLIED MATERIALS LIST IS INCLUDED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUPPLYING ALL ADDITIONAL SHEET PILE. SHOULD THE MANUFACTURER OF THE CONTRACTOR SUPPLIED SHEET PILE DIFFER FROM THE OWNER SUPPLIED MATERIALS, A MAKE-UP SHEET, AS DETAILED IN THE PLANS, SHALL BE USED IN ALL AREAS WHERE SHEETS FROM DIFFERING MANUFACTURERS CONNECT

# FLAT SHEET PILE FABRICATION

WYE AND ANCHOR PILES SHALL BE FABRICATED SUCH THAT THE FINISHED SHEET PILE CAMBER DOES NOT VARY BY MORE THAN 1/8 INCH IN ANY 10-FEET LENGTH, AND THE FINISHED SHEET PILE SWEEP DOES NOT VARY BY MORE THAN 1/8 INCH IN ANY 5-FEET LENGTH. CONTROL HEAT INPUT TO MINIMIZE DISTORTION. HEAT STRAIGHTEN PER ÁSIC AS REQUIRED. FABRICATOR SHALL SUBMIT WYE PILE FABRICATION AND QUALITY CONTROL PLAN.

### FLAT SHEET PILE HANDLING

SHEET PILE SHALL BE STORED AND HANDLED PER MANUFACTURER RECOMMENDATIONS TO PREVENT DEFLECTION, DISTORTION AND DAMAGE TO THE PILING. PILES SHALL BE LOADED, TRANSPORTED AND UNLOADED IN A HORIZONTAL POSITION, AS SINGLE UNITS OR IN BUNDLES UP TO FOUR UNITS ACCORDING TO THE PILE LENGTH AND THE LIFTING CAPACITY OF THE HOISTS. PILES UP TO 40-FEET IN LENGTH SHALL BE TRANSPORTED BY LIFTING AT LEAST TWO POINTS. THE POINTS SHALL BE NOT MORE THAN 16 FEET FROM THE ENDS OF THE PILES. SHEET PILE MAY BE STACKED ON TOP OF EACH OTHER PROVIDED THEY ARE OFFSET SIDEWAYS SO THAT THE INTERLOCKS ARE SITUATED ALTERNATELY IN THE SAME VERTICAL PLANE. STACKING OF SHEET PILE SHALL BE SEPARATED BY SQUARE TIMBER PLACED PER MANUFACTURER ALONG THE LENGTH OF THE PILE TO LIMIT DEFORMATION. THE END OVERHANG SHALL NOT BE OVER 5 FEET, SPACING OF TIMBERS SHALL NOT BE OVER 10 FEET.

# OPEN CELL FLAT SHEET PILE DRIVING

CONTRACTOR'S PILE DRIVING SUPERINTENDENT OR PROJECT MANAGER SHALL HAVE EXPERIENCE WITH INSTALLATION OF AT LEAST 3 OPEN CELL OR CLOSED CELL BULKHEADS OF SIMILAR MAGNITUDE WITHIN THE LAST SEVEN (7) YEARS. SUBMIT QUALIFICATIONS AND RESUME PRIOR TO MOBILIZATION.

SHEET PILES SHALL BE DRIVEN FULL LENGTH WITH A VIBRATORY AND/OR IMPACT HAMMER BY METHODS WHICH WILL ACHIEVE PENETRATION WITHOUT PILE DAMAGE. ALL SHEET PILE DRIVING METHODS AND EQUIPMENT SHALL BE SUBMITTED TO THE ENGINEER AND APPROVED BEFORE DRIVING STARTS. ALL DRIVING SHALL BE DONE WITH THE ENGINEER PRESENT.

PILES SHALL BE DRIVEN SUCH THAT THE TIPS OF ADJACENT PILES DO NOT VARY MORE THAN 5-FOOT. NO FACE SHEET PILE IN ANY CELL UNIT SHALL BE DRIVEN MORE THAN 5-FOOT BEYOND ANY OTHER SHEET IN THAT CELL UNIT. IN INSTANCES OF HARD DRIVING, THIS DISTANCE SHALL BE REDUCED TO 2 FT. HARD DRIVING OF SHEET PILE SHALL BE DEFINED AS A PILE ADVANCEMENT RATE OF LESS THAN 6-INCHES PER MINUTE OR WHEN INTERLOCKS BECOME OVERHEATED (150°F). ADDITIONAL ASSISTIVE METHODS SUCH AS: CHANGING OF VIBRATORY HAMMER FREQUENCY, PILE PROBING, PRE-DRILLING, EXCAVATION, OR WATER JET MAY BE REQUIRED DURING HARD DRIVING. THE ENGINEER SHALL BE CONTACTED IMMEDIATELY IF HARD DRIVING IS ENCOUNTERED

IT IS RECOMMENDED THAT A SHEET PILE THREADER (STAB CAT® OR SIMILAR) BE USED DURING THREADING OF ALL SHEET PILE. SNIPING OF SHEET PILE SHALL ONLY OCCUR AT THE TOP OF THE SHEET PILE WITHIN THE PORTION OF SHEET ABOVE FINAL CUTOFF ELEVATION PER THE PROVIDED DETAIL. TEMPORARY WELDS SHALL NOT BE PERFORMED ON SHEET PILE INTERLOCKS WITHOUT ENGINEER APPROVAL TEMPORARY WELDS LOCATED ON THE SHEET PILE SHALL BE REMOVED BY GRINDING AND SHALL BE FLUSH WITH THE ORIGINAL SURFACE. TEMPORARY WELDS SHALL NOT BE INTENTIONALLY BROKEN (I.E. DRIVEN THROUGH) DURING SHEET PILE DRIVING. COATING SHALL BE REPAIRED WHERE THEY WILL BE EXPOSED AFTER CONSTRUCTION IS COMPLETE.

CONTRACTOR SHALL PROVIDE A REGISTERED SURVEYOR TO CONFIRM CELL LAYOUT PRIOR TO INSTALLING SHEET PILE AND PROVIDE PERIODIC CHECKS OF LAYOUT AND ALIGNMENT DURING INSTALLATION.

# FLAT SHEET PILE INSTALLATION TOLERANCE

FACE AND ENDWALL SHEET PILES AND SHALL BE DRIVEN USING A TEMPLATE. CONTRACTOR SHALL SUBMIT TEMPLATE SHOP DRAWINGS FOR REVIEW PRIOR TO DRIVING SHEETS. FACE SHEET PILES SHALL NOT BE DRIVEN MORE THAN 3 INCHES FROM PLAN LOCATION AT CUTOFF ELEVATION. NOR MORE THAN A 1/4-INCH-PER-FOOT LENGTH OUT OF PLUMB IN ANY DIRECTION. FACE AND ENDWALL SHEETS SHALL BE DRIVEN AND LEFT 1 FOOT ABOVE PLANNED CUT-OFF FLEVATION AND MONITORED AS DESCRIBED BELOW BEFORE CUT-OFF.

TAILWALL SHEET PILES SHALL BE DRIVEN IN A STRAIGHT LINE OR SMOOTH CURVE AS SHOWN, WITH PILES NOT MORE THAN 2 FEET FROM PLAN LOCATION, NOR MORE THAN A 1/2-INCH PER FOOT LENGTH OUT OF PLUMB IN ANY DIRECTION. DRIVING OF TAILWALL SHEETS SHALL BE DRIVEN FROM THE MOST CONSTRAINED END TOWARDS THE FREE END (I.E. FROM WYE TOWARDS ANCHOR).

WYE PILES SHALL BE DRIVEN NOT MORE THAN 2 INCHES FROM PLAN LOCATION AT THE TOP, AND NOT MORE THAN 1/4-INCH PER FOOT OF LENGTH OUT OF PLUMB. THE PLAN DISTANCE FROM CENTERLINE TO CENTERLINE OF WYE SECTIONS AT THE TOP SHALL BE MAINTAINED WITH RIGID BRACING, AFTER DRIVING AND DURING FILL OPERATIONS, THE PLAN WYE DRIVING LOCATION SHALL BE DETERMINED IN CONSULTATION WITH THE ENGINEER AS THE COMPLETED CELLS. ARE EXPECTED TO MOVE OUTWARDS 6 INCHES OR MORE AS THE CELL EXPANDS.

# OBSTRUCTIONS

IF OBSTACLES ARE ENCOUNTERED ALONG THE CELL FACE THAT WOULD INTERFERE WITH SHEET DRIVING. THE DEBRIS SHALL BE EXCAVATED, REMOVED, AND THE SUBSEQUENT VOID REFULED, IF OBSTACLES ARE ENCOUNTERED ALONG THE TAIL WALL, THE DEBRIS WILL BE REMOVED AS PREVIOUSLY STATED, OR THE WALL ALIGNMENT SHALL BE CURVED AWAY FROM THE OBSTACLE IN A SMOOTH CURVE AS APPROVED BY THE ENGINEER. IN SUCH EVENTS, THE ENGINEER SHALL BE CONTACTED IN ADVANCE OF REMOVING THE OBSTACLE OR REALIGNING THE TAILWALL, SHOULD SOFT SOILS BE ENCOUNTERED, FACE SHEETS MAY REQUIRE SUPPORT FROM THE TEMPLATE BEFORE FILLING CELL. CONTRACTOR'S SHALL DEVELOP METHOD NECESSARY TO EMBED SHEET PILES AS IDENTIFIED IN THE DRAWINGS.

### POST DRIVING

AFTER DRIVING ALL SHEETS WITHIN A CELL TO REQUIRED ELEVATION AND PRIOR TO BACKFILLING, THE CONTRACTOR SHALL PULL AND RE-DRIVE A MINIMUM OF 10% OF THE SHEETS WITHIN THE CELL TO VERIFY PROPER INTERLOCK CONNECTION AND ALIGNMENT BETWEEN SHEETS. THE SHEET PILE SHALL BE PULLED TO AN ELEVATION SUCH THAT 15-FOOT +/- 2 FOOT OF THE SHEET REMAINS THREADED BETWEEN THE ADJACENT SHEETS. PULLING AND RE-DRIVING OF SHEETS SHALL BE PERFORMED WITH THE OWNER'S REPRESENTATIVE PRESENT.

#### TIDAL VARIATION

TIDAL VARIATION WILL AFFECT SHEET PILE CONSTRUCTION. RESULTING POTENTIAL DIFFERENTIAL WATER HEADS INSIDE AND OUTSIDE CELL CONSTRUCTION MUST BE MINIMIZED. TO ACCOMPLISH THIS, HOLES UP TO 12 INCHES IN DIAMETER MAY BE CUT THROUGH TAIL WALLS AT LOCATIONS APPROVED BY THE ENGINEER.

## OPEN CELL SHEET PILE FILLING

FILL WITHIN THE SHEET PILE CELLS SHALL CONSIST OF MATERIALS AS INDICATED IN THE PLANS. THE INITIAL FILL FROM EXISTING MUDLINE TO ELEVATION +5 MILLW SHALL NOT BE DUMPED INTO FINAL POSITION, BUT SHALL BE DUMPED ON TOP OF THE EMBANKMENT AND PUSHED INTO PLACE IN A MANNER THAT WILL INSURE PROPER PLACEMENT SUCH THAT VOIDS, POCKETS, SEGREGATION AND BRIDGING WILL BE REDUCED TO A MINIMUM.

FILL SHALL BE PLACED IN 12-INCH-THICK MAXIMUM HORIZONTAL LIFTS ABOVE FLEVATION +5 MILW. FACH LIFT SHALL BE COMPACTED BY NO LESS THAN 5 PASSES OF A 10-TON VIBRATORY ROLLER. SMALLER COMPACTORS AND ADDITIONAL CARE SHALL BE USED TO COMPACT WITHIN 5 FEET OF THE DOCK FACE SHEET PILES TO PREVENT DAMAGE OR DISTORTION TO THE BULKHEAD FACE. SPECIAL CARE SHALL ALSO BE USED TO OBTAIN THOROUGH COMPACTION AGAINST TAIL WALL SHEET PILES.

FILL SHALL BE PLACED AS FOLLOWS AROUND SHEET PILE CELLS TO PREVENT DISTORTION OF THE BULKHEAD. ALL FACE AND ADJACENT TAIL WALL SHEETS IN A CELL SHALL BE INSTALLED TO REQUIRED ELEVATION PRIOR TO PLACING FILL AGAINST THE FACE OF THE CELL, PLACE FILL IN APPROXIMATELY LEVEL LIFTS ACROSS THE ENTIRE CELL AREA. FILL AROUND TAIL WALL SHEFTS FIRST THEN FILL AGAINST FACE SHEFTS THE FLEVATION OF FILL BETWEEN ADJACENT CELLS SHALL NOT DIFFER BY MORE THAN 3 FEET. THE CONTRACTOR SHALL USE RIGID BRACING BETWEEN WYES THAT SHALL BE INSTALLED PRIOR TO CELL FILLING THE CONTRACTOR IS CAUTIONED THAT UNEVEN FILLING OF CELLS OR FAILURE TO MAINTAIN PLAN DISTANCE BETWEEN WYES WILL RESULT IN UNDESIRABLE DISTORTIONS OF THE SHEET PILE WALL WHICH THE CONTRACTOR SHALL BE REQUIRED TO CORRECT.

MATERIAL THAT IS LOST TO TIDE OR WAVE ACTION SHALL NOT BE CONSIDERED A PAY ITEM BUT SHALL BE INCIDENTAL TO THE CONTRACT SUM.

#### OPEN CELL BULKHEAD STABILIZATION

DURING AND AFTER FILLING, THE BULKHEAD FACE IS EXPECTED TO MOVE 6 INCHES OR MORE OUTWARD AND TO SETTLE VERTICALLY. AFTER FILLING TO WITHIN 2 FOOT OF FINISHED GRADE, THE FILLING SHALL BE DISCONTINUED AND VERTICAL AND HORIZONTAL MOVEMENT OF THE CELLS WILL BE MEASURED BY THE CONTRACTOR EVERY THREE DAYS UNTIL THE CELL SETTLEMENT HAS STABILIZED.

CELL SETTLEMENT MAY BE CONSIDERED STABILIZED WHEN SHEET PILE WYE DIRECTIONAL MOVEMENT (HORIZONTAL AND/OR VERTICAL) RATES SLOW TO A 7-DAY AVERAGE OF 0.05 FOOT OR LESS PER WEEK (7 DAYS) AS DETERMINED BY THE ENGINEER. AFTER STABILIZATION, SHEET PILE CUTOFF, SHEET PILE INTERLOCK WELDING, BOARDWALK FACE BEAM INSTALLATION AND FILLING TO FINISH ELEVATION CAN BEGIN. ADDITIONAL GRADING MAY BE REQUIRED TO COMPENSATE FOR BULKHEAD SETTLEMENT AND SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT SUM.

# BOARDWALK FACE BEAM ALIGNMENT

AFTER CELL SETTLEMENT HAS STABILIZED AS DEFINED IN "OPEN CELL BULKHEAD STABILIZATION", THE CONTRACTOR SHALL PROVIDE AN AS-BUILT SURVEY OF THE CONSTRUCTED SHEET PILE DOCK SPECIFICALLY IDENTIFYING ALL FINAL WYE LOCATIONS AND THE SEAWARD CELL APEX OF EACH CELL TO THE ENGINEER. A FINAL BOARDWALK FACE ALIGNMENT SHALL BE ESTABLISHED AND APPROVED BY THE ENGINEER TO WHICH THE BOARDWALK FACE BEAM SHALL BE ALIGNED. THE BOARDWALK FACE BEAM SHALL BE INSTALLED AFTER A FINAL DOCK FACE ALIGNMENT HAS BEEN ESTABLISHED BY THE ENGINEER.

STRUCTURAL STEEL MISCELLANEOUS PLATES AND SHAPES SHALL BE ASTM A36 OR A572 GR. 50, GALVANIZED, UNLESS OTHERWISE NOTED. PIPE SHALL BE ASTM A53, GRADE B, TYPE E OR S, GALVANIZED, UNLESS OTHERWISE NOTED. TUBES SHALL BE ASTM A500, GRADE B, GALVANIZED, UNLESS OTHERWISE NOTED.

STEEL WELDING PER LATEST AWS D1.1 BY WELDERS QUALIFIED PER AWS FOR THE TYPE AND POSITION OF THE WELDS WELDED. ALL FILLER METAL SHALL MEET CHARPY IMPACT CRITERIA OF 20 FT-LBS AT -20 DEGREES FAHRENHEIT AND SHALL HAVE A MAXIMUM CARBON CONTENT OF 0.20%. ALL SMAW ELECTRODES SHALL BE PROPERLY CONDITIONED LOW HYDROGEN. SUBMIT WELDER QUALIFICATIONS AND WELDING PROCEDURES TO ENGINEER FOR APPROVAL AT LEAST 15 DAYS PRIOR TO WELDING.

ALL WELDS SHALL BE 100% VISUALLY INSPECTED BY THE CONTRACTOR. IN ADDITION 10% OF ALL CJP SHOP WELDS SHALL BE TESTED BY UT EXAMINATION OR OTHER NDT METHODS APPROVED BY ENGINEER. ALL FIELD WELDS WILL BE 100% VISUALLY INSPECTED BY THE OWNER. ANY WELD FAILING INSPECTION SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE. WHICH WILL INCLUDE THE COST FOR RETESTING.

NO WELDING THROUGH GALVANIZED COATING WILL BE PERFORMED. THE GALVANIZING WITHIN ONE INCH OF THE WELD SHALL BE REMOVED AND REPAIRED AFTER WELDING.

BOLTS

IF "NYLOCK" NUTS ARE SPECIFIED WITH STEEL BOLTS, THE BOLT SHALL BE THREADED AND GALVANIZED TO ACCEPT THE CORROSION RESISTANT NYLOCK NUT.

ALL OTHER BOLTS, THRU-RODS, LAG SCREWS, NUTS, WASHERS, NAILS AND SPIKES SHALL MEET ASTM A307 OR ASTM A36 OR SIMILAR REQUIREMENTS AS APPROVED BY THE ENGINEER AND SHALL BE HOT-DIPPED GALVANIZED. WASHERS SHALL BE USED UNDER BOTH HEAD AND NUT OF ALL ASTM A307 AND ASTM A36 BOLTS AND THRU-RODS AND UNDER THE HEAD OF ALL ASTM A307 AND ASTM A36 LAG SCREWS. MALLEABLE IRON WASHERS SHALL BE USED IN ALL AREAS WHERE THE BOLT HEAD OR NUT SHALL BEAR AGAINST WOOD, EXCEPT UNDER ECONOMY HEADS.

### GALVANIZING

# GALVANIZING/METALIZING REPAIR

CONTRACTOR SHALL TAKE NECESSARY MEANS TO PROTECT COATINGS DURING TRANSPORTATION, HANDLING, WELDING, CUTTING AND INSTALLATION. DAMAGED GALVANIZING INCLUDING THAT REMOVED FOR WELDING, WELDS, CUTS, GOUGES OR OTHER HOLIDAYS IN THE COATINGS SHALL BE REPAIRED BY THE CONTRACTOR.

SHOP REPAIR OF GALVANIZING/METALIZING SHALL BE DONE BY MEANS OF SPRAY METALLIZING. FIELD REPAIR DAMAGED GALVANIZING BY SPRAY METALLIZING IF OVER 100 SQUARE INCHES BY SPRAY METALLIZING. "GALV STICK" OR ENGINEER APPROVED EQUAL MAY BE USED FOR FIELD REPAIR UNDER 100 SQUARE INCHES. CONTRACTOR SHALL SUBMIT REPAIR MATERIALS AND METHODS OF REPAIRS TO ENGINEER FOR REVIEW AND APPROVAL.

GALV-STICK

GALV-STICK SHALL BE ZINC OR ALUMINUM ALLOY. PREPARE DAMAGED GALVANIZING WITH A GRINDER AND THEN ABRADE THE ENTIRE SURFACE WITH A WIRE BRUSH WHERE APPLICATION OF THE GALVANIZING REPAIR IS REQUIRED. CLEAN THE SURFACE TO REMOVE ALL GREASE, OIL, AND SURFACE DEPOSITS. HEAT LOCAL AREA TO MANUFACTURER SUGGESTED TEMPERATURE AND APPLY GALV-STICK IN MANNER TO ACHIEVE MINIMUM 10 MIL TOTAL FINAL THICKNESS. AFTER COOLING, APPLY 2 COATS OF ZINC-RICH PAINT. ALLOW EACH COATING TO DRY THOROUGHLY BETWEEN APPLICATIONS.

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ALL BOLTS CONNECTING STEEL TO CONCRETE OR STEEL TO STEEL CONNECTIONS SHALL BE ASTM A325. GALVANIZED, UNLESS OTHERWISE NOTED. WASHERS SHALL BE USED UNDER BOTH HEAD AND NUT OF ALL ASTM A325 BOLTS. ALL STAINLESS STEEL (S.S.) FASTENER COMPONENTS SHALL BE TYPE 316 S.S.

ALL STEEL, PILE AND HARDWARE SHALL BE HOT-DIPPED GALVANIZED PER ASTM A385 A123 OR A153 AFTER FABRICATION UNLESS OTHERWISE NOTED. (DAMAGED GALVANIZING, INCLUDING THAT REMOVED FOR WELDING SHALL BE REPAIRED BY SPRAY METALLIZING).

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# WHITTIER SMALL BOAT HARBOR **IMPROVEMENTS - PHASE I**

**GENERAL NOTES** (2 OF 4)

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# **GENERAL NOTES** (cont.)

# SPRAY METALLIZING

ALL SPRAY METALLIZING SHALL BE PERFORMED AWS C2.23-2003. STEEL SUBSTRATE SHALL BE PREPARED TO SSPC-SP/NACE NO 1 WHITE METAL BLAST FINISH WITH A MINIMUM ANGULAR PROFILE DEPTH OF 2.5. MILS. BLAST MEDIA SHALL BE KLEEN BLAST SIZE 16-30 AS MANUFACTURED BY KLEEN INDUSTRIAL SERVICES (800-227-1134) OR ENGINEER APPROVED EQUAL. AFTER BLASTING REMOVE DUST AND SPENT ABRASIVE FROM THE SURFACE BY USING OIL-FREE PRESSURIZED AIR, BRUSHING, OR VACUUM CLEANING. THE STEEL SURFACE TEMPERATURE SHALL BE AT LEAST 5 DEGREES F ABOVE THE DEW POINT OF THE AMBIENT AIR TEMPERATURE. FOR FLAMESPRAYING THE INITIAL STARTING AREA SHALL BE PREHEATED TO 250 DEGREES F. FEEDSTOCK SHALL BE 85/15 ALUMINUM/ZINC APPLIED IN SEVERAL PASSES (APPROXIMATELY 2-4 MILS/PASS) TO A MINIMUM DRY COATING FILM THICKNESS OF 12 MILS. DURING APPLICATION, SPRAY GUN SHALL BE HELD PERPENDICULAR TO THE SUBSTRATE AT A STAND-OFF DISTANCE OF 6 TO 10 INCHES. THE CONTRACTOR SHALL PERIODICALLY VERIFY PASS AND TOTAL COATING THICKNESSES. TENSILE BOND STRENGTH SHALL MEASURED PER ASTM D4541 AT THE START OF EACH SHIFT, AFTER ANY CHANGE TO THE APPLICATION METHOD, OR EVERY 500 SF. THE MINIMUM TENSILE BOND SHALL BE 700 PSI. FIELD METALLIZING SHALL BE BOND TESTED BY AN OWNER'S REPRESENTATIVE WITH THE CONTRACTOR'S ASSISTANCE. CONTRACTOR SHALL SUBMIT METALLIZING EQUIPMENT, BLAST MEDIA, FEEDSTOCK MATERIAL CERTIFICATION, APPLICATION AND QUALITY CONTROL METHOD FOR ENGINEER REVIEW AND APPROVAL.

## RUBBER

RUBBER BUSHINGS FOR PIANO HINGE CONNECTIONS SHALL BE NATURAL RUBBER COMPOUND REINFORCED WITH CARBON BLACK. HARDNESS SHALL BE 70 DUROMETER SHORE A. MINIMUM TENSILE STRENGTH OF 2000 PSI AS PER ASTM D412. ULTIMATE ELONGATION AT BREAK OF 300% AS PER ASTM D412. OPERATING TEMPERATURE SHALL BE -40° TO 158° FAHRENHEIT. ALL OTHER RUBBER COMPONENTS SHALL BE BUTYL RUBBER WITH 50 DUROMETER HARDNESS WITH TEMPERATURE RATING TO -20 DEGREES F.

FIT-UP OF EACH PIPE HINGE CONNECTION IS CRITICAL. PIPE HINGE RUBBER BUSHINGS MUST FIT INTO PIPE SEGMENTS, YET NOT ALLOW APPRECIABLE MOVEMENT WITHIN THE PIPE. FABRICATOR SHALL COORDINATE WITH BUSHING MANUFACTURER AND METAL FABRICATOR TO PRODUCE SAMPLE BUSHINGS AND A SAMPLE HINGE ASSEMBLY, COMPLETE WITH GALVANIZED COATING, AND USING PIPE FROM THE SINGLE BATCH OF PIPE PROPOSED TO BE USED FOR THE PROJECT. THIS SAMPLE ASSEMBLY SHALL BE USED TO DETERMINE FINAL BUSHING OUTSIDE DIAMETER REQUIRED TO ACHIEVE A FIT THAT IS SNUG-TIGHT WITHIN THE GALVANIZED PIPE. BUSHING DIMENSIONS INDICATED ON THE PLANS ARE FOR INITIAL FABRICATION ONLY. BUSHINGS MAY NEED TO THEN BE TURNED DOWN, ON A LATHE, TO ACHIEVE THE FINAL FIT-UP REQUIREMENTS. A SAMPLE BUSHING OF THE DIMENSIONS AS DETERMINED BY THE METHOD DESCRIBED ABOVE. COMPLETE WITH THE REQUIRED, PRESSED-FIT UHMW SLEEVE, SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION OF BUSHINGS TO BE USED FOR THIS PROJECT.

# TIMBER PRESSURE TREATMENT

ALL SAWN TIMBER, GLULAMINATED TIMBER, PLYWOOD, AND TIMBER PILES SHALL BE INCISED AND PRESSURE TREATED ACCORDING TO CURRENT AWPA SPECIFICATIONS TO THE FOLLOWING RETENTIONS:

1) SAWN TIMBER AND PLYWOOD LOCATED ABOVE WATERLINE SHALL HAVE A NET DRY SALT RETENTION OF NOT LESS THAN 0.60 POUNDS PER CUBIC FOOT OF ACZA (AMMONIACAL COPPER ZINC ARSENATE) IN THE ASSAY ZONE PER AWPA C18.

2) SAWN TIMBER LOCATED BELOW WATERLINE SHALL BE TREATED TO A MINIMUM RETENTION OF CREOSOTE OF 20 PCF PER AWPA C18.

3) GLULAMINATED TIMBERS SHALL BE TREATED TO A MINIMUM RETENTION OF CREOSOTE OF 12 PCF PER

ALL TIMBER MEMBERS BEING MANUFACTURED SHALL BE PRODUCED IN ACCORDANCE WITH THE CURRENT INDUSTRY BEST MANAGEMENT PRACTICES (BMP) SET FORTH BY THE WESTERN WOOD PRESERVERS INSTITUTE (WWPI).

THE CONTRACTOR SHALL HIRE AN INDEPENDENT AGENCY CERTIFIED BY THE AMERICAN LUMBER STANDARD COMMITTEE TO INSPECT TIMBER MEMBERS. INSPECTION PROCEDURES SHALL BE PERFORMED IN ACCORDANCE TO AWPA M2. THE INSPECTOR SHALL REPORT ALL FINDINGS DIRECTLY TO THE OWNER'S REPRESENTATIVE ON THE DAY OF INSPECTION. THE OWNER RESERVES THE RIGHT TO DO ADDITIONAL TESTING AT OWNER'S OWN EXPENSE

THE MANUFACTURER SHALL NOTIFY THE INSPECTOR OF ALL PRESSURE TREATING DAYS AND TIMES A MINIMUM OF 24-HOURS IN ADVANCE. THE MANUFACTURER WILL AID THE INSPECTOR WITH OBTAINING SAMPLE TIMBERS AND PROVIDING LOCATION FOR TESTING

EXCESSIVE EXTERIOR CREOSOTE RESIDUE, AS DICTATED BY THE INDUSTRY BMP, SHALL NOT BE ALLOWED AND MUST BE REMOVED AT NO ADDITIONAL COST TO THE OWNER. EXCESSIVE INCISING THAT CAUSES STRUCTURAL DAMAGE TO THE TIMBER SHALL BE REJECTED AND REPLACED AT NO ADDITIONAL COST TO THE OWNER. "EXCESSIVE INCISING" IS DEFINED AS UNDUE INCISION DEPTHS AND INCISION DENSITY PER AREA OF TIMBER WHEN PRESERVATIVE RETENTION HAS BEEN ACHIEVED. EVIDENCE OF EXCESSIVE INCISING SHALL BE TIMBER FLAKING OR CHIPPING DUE TO LIGHTLY SCUFFING THE TIMBER SURFACE. IN NO CASE SHALL INCISING EXCEED THE MAXIMUMS ALLOWED BY NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION, SECTION

TIMBER COMPONENTS SHALL BE CUT TO LENGTH DRILLED DAPPED AND SHAPED REFORE PRESSURE TREATING. ANY FIELD FABRICATION OR DAMAGE SHALL BE REPAIRED PER AWPA M4.

## CONCRETE

CEMENT SHALL CONFORM TO ASTM C150 TYPE II, OR TYPE I OR III WITH TRI-CALCIUM ALUMINATE CONTENT BELOW 8%. AGGREGATE SHALL CONFORM TO ASTM C33 WITH MAXIMUM SIZE OF 3/4 IN. CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 6,000 PSI. A MINIMUM OF 35 LBS/CY OF SILICA FUME SHALL BE ADDED TO CONCRETE. ENTRAINED AIR SHALL BE 4 TO 7%. MAXIMUM WATER-CEMENTITIOUS MATERIALS RATIO SHALL BE 0.40. MATERIALS, BATCHING, MIXING AND TRANSPORTATION SHALL BE IN ACCORDANCE WITH ASTM C94. MIX DESIGN AND TESTING SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 AND ACI 345. MIXING, FORMWORK, REINFORCEMENT, CONCRETE PLACEMENT, CONSOLIDATION, FINISHING AND CURING SHALL FOLLOW THE STANDARDS SET BY ACL 345.

THE CONTRACTOR SHALL SUBMIT A DETAIL FOR PANEL LIFTING MECHANISM. LIFTING MECHANISM SHALL BE LOCATED BELOW THE UPPER LAYER OF REINFORCEMENT. LIFTING MECHANISM SHALL NOT PROTRUDE FROM FINAL, IN-PLACE FINISH SURFACES OF CONCRETE.

## CONCRETE QUALITY CONTROL

AIR ENTRAINMENT TESTS AND TEMPERATURE TESTS SHALL BE TAKEN BY AN ACI TECHNICIAN FROM EACH CONCRETE TRUCK, OR EVERY 10 CY OF CONCRETE WHERE TRUCKS ARE NOT UTILIZED. COMPRESSIVE STRENGTH TEST CYLINDERS SHALL ONLY BE TAKEN AFTER ALL ADDITIVES HAVE BEEN ADDED. A MINIMUM OF THREE (3) COMPRESSIVE TEST CYLINDERS WILL BE TAKEN DAILY, PER MIX. CONTRACTOR SHALL NOTIFY ACI TECHNICIAN AND OWNER'S REPRESENTATIVE OF ALL CASTING DAYS AND TIMES A MINIMUM OF 24-HOURS IN ADVANCE, CONTRACTOR SHALL AID THE ACI TECHNICIAN WITH OBTAINING SAMPLES AND PROVIDING A LOCATION FOR CURING AND TESTING. AT NO TIME WILL WATER BE ADDED TO CONCRETE IN THE FIELD TO INCREASE WORKABILITY

# REINFORCING STEEL

ALL REINFORCING SHALL BE NEW BILLET STOCK GALVANIZED ASTM A706, GRADE 60 STEEL UNLESS NOTED OTHERWISE. BARS SHALL BE SUPPORTED ON APPROVED CHAIRS OR WELL-CURED CONCRETE BLOCKS. REINFORCING STEEL SHALL BE DETAILED, BENT, AND PLACED IN ACCORDANCE WITH THE LATEST ACI 318. TWO-INCH MINIMUM CLEARANCE UNLESS OTHERWISE NOTED. BARS SHALL BE CLEAN AND FREE FROM CUTTING OIL OR OTHER DELETERIOUS MATERIAL.

NON-SKID COATING ALL STEEL WALKING SURFACES ON THE FLOATS & WALKWAYS SHALL BE PROVIDED WITH A NON-SKID COATING, A NON-SKID COATING SHALL CONSIST OF THE SPECIFIED YELLOW GRIT PAINT OR A ROUGH SPRAY METALLIZED SURFACE. ALL FLAT, TOP SURFACES OF ALL CONNECTION ASSEMBLIES SHALL BE COATED WITH "SURE-GRIP", SAFETY YELLOW, TO A MINIMUM 60-MIL DFT. SURE-GRIP IS AVAILABLE FROM FARWEST PAINT MANUFACTURING COMPANY IN TUKWILA, WA. THE COATING SHALL BE APPLIED IN THE SHOP, SPECIFICALLY PER MANUFACTURER'S RECOMMENDATIONS. FINISH SHALL HAVE NAPE THAT PROVIDES ADEQUATE NON-SKID CHARACTERISTICS ACCEPTABLE TO INDUSTRY STANDARDS. DAMAGED FINISH SHALL BE CAUSE FOR REJECTION. SUPPLIER SHALL PROVIDE SAMPLES OF COATED, GALVANIZED STEEL PLATE w/ VARIOUS ROUGHNESS TO ENGINEER FOR APPROVAL.

BOLLARD PAINT FINISH PROTECTIVE BOLLARDS SHALL BE PAINTED WITH TWO 4-MIL COATS OF ZINC OXIDE PAINT. TOP COAT SHALL BE "CATERPILLAR YELLOW" OR OTHER SUITABLE BRIGHT SAFETY YELLOW.

#### ARMOR ROCK

ARMOR ROCK SHOULD BE HARD (UNCONFINED COMPRESSIVE STRENGTH GREATER THAN 10,000 PSI ASTM D-2938), ANGULAR, AND HAVE NO MORE THAN 30% LOSS AT 1000 REVOLUTIONS, AS DETERMINED BY ASTM C-535. BULK SPECIFIC GRAVITY NOT LESS THAN 2.60 (AASHTO T 104), WATER ABSORPTION NOT MORE THAN 2.5% (WAQTC FOP FOR AASHTO T 85), AND BREADTH AND THICKNESS AT LEAST 1/3 ITS LENGTH. ROUNDED ROCK WILL NOT BE ACCEPTED. STONES SHALL ALSO ENDURE FREEZE/THAW (25 CYCLES) HAVING LESS THAN 5% LOSS (ASTM C-666), SODIUM SULFATE SOUNDNESS (5 CYCLES) HAVING LESS THAN 12% LOSS (ASTM C-88), AND EXPANSIVE BREAKDOWN IN ETHYLENE GLYCOL (15 DAYS) HAVING LESS THAN 8% LOSS (CRD-C148).

ARMOR ROCK SHOULD BE WELL GRADED WITH WEIGHTS OF INDIVIDUAL STONES RANGING FROM AT LEAST 30 LBS TO 400 LBS MAX. AT LEAST 50 PERCENT OF THE INDIVIDUAL STONES SHALL WEIGH MORE THAN 100 LBS. THE ARMOR LAYER SHALL BE A MINIMUM OF 3.0 FEET THICK AND AT LEAST THREE (3) STONES THICK. STONE SHALL BE PLACED SUCH THAT A WELL KEYED, STABLE ROCK MASS WITH A RELATIVELY REGULAR SURFACE IS OBTAINED. PLACING STONE THROUGH CHUTES, DROPPING STONE MORE THAN 4 FEET, AND OTHER METHODS WHICH SEGREGATE OR DAMAGE THE STONE WILL NOT BE PERMITTED. THE FINISHED REVETMENT SURFACE SHALL BE FREE FROM POCKETS OF STONE LESS THAN AVERAGE SIZE. INDIVIDUAL STONES SHALL NOT PROTRUDE MORE THAN 12 INCHES ABOVE THE AVERAGE LEVEL OF THE SLOPE.

BULKHEAD FILL

LEVELING COURSE

GEOTEXTILE FABRIC GEOTEXTILE SHALL BE NONWOVEN GEOTEX 1071 AS MANUFACTURED BY PRONEX ON APPROVED EQUAL. GEOTEXTILE SHALL BE LAID OUT PARALLEL TO SLOPE SUCH THAT ADJACENT SHEETS OVERLAP A MINIMUM OF 24-INCHES. INSTALL PER MANUFACTURES RECOMMENDATIONS.

TOPSOIL AND SEEDING TOPSOIL AND SEEDING SHALL BE PAID LUMP SUM. WORK SHALL INCLUDE TOPSOIL, SEED, MULCH, FERTILIZER, AND WATERING AS NECESSARY TO ESTABLISH A PERMANENT VEGETATIVE MAT. SEEDING SHALL BE COMPLETED BETWEEN MAY 15TH AND AUGUST 15TH.

FURNISH TOPSOIL THAT IS REPRESENTATIVE OF THE EXISTING NATURAL ORGANIC BLANKET OF THE LOCAL AREA. SOIL WITH A MINIMUM OF 5% ORGANIC CONTENT, AS DETERMINED BY ATM 203, MAY BE USED AS TOPSOIL. REMOVE ROOTS, STUMPS, UNNATURAL MATERIAL, AND ROCKS GREATER THAN 3 INCH IN DIAMETER.

PRIOR TO SEEDING, TOPSOIL SHALL BE TRACK WALKED, OR OTHERWISE PREPARED, TO PROVIDE GROOVES PERPENDICULAR TO THE DIRECTION OF THE SLOPE.

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BULKHEAD FILL SHALL BE FREE OF ORGANICS, ICE, SNOW, AND OTHER DELETERIOUS MATERIALS. SUITABLE NON-CONTAMINATED DREDGE SPOILS MAY BE USED AS BULKHEAD FILL. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ADDITIONAL FILL SOURCES AS NECESSARY. MAX FINES = 8%.

LEVELING COURSE SHALL MEET MOA SPECIFICATIONS FOR LEVELING COURSE MATERIAL. AC PAVEMENT-SHALL MEET MOA SPECIFICATIONS FOR CLASS-E ASPHALT CONCEPTS.

SEED MIX SHALL BE 50% 'NORCOAST' BERING HAIRGRASS, 40% 'ARCTARED' RED FESCUE, AND 10% 'NORTRAN' TUFTED HAIRGRASS, CONTRACTOR MAY SUBMIT ALTERNATE SEED MIXES THAT ADHERE TO THE RECOMMENDATIONS FROM "REVEGETATION MANUAL FOR ALASKA" PUBLISHED BY THE DEPARTMENT OF NATURAL RESOURCES. SEED SHALL BE APPLIED AT 0.7 LBS PER 1,000 SQUARE FEET. FERTILIZER WITH AN N-P-K OF 20-20-10 SHALL BE APPLIED AT A RATE OF 12.0 LBS PER 1,000 SQUARE FEET.

APPLY SEED WITH A BONDED FIBER MATRIX MULCH SUCH AS "FLEXTERRA" OR APPROVED EQUAL. MULCH SHALL BE APPLIED A RATE OF 70.0 LBS PER 1.000 SQUARE FEET.

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# WHITTIER SMALL BOAT HARBOR **IMPROVEMENTS - PHASE I**

**GENERAL NOTES** (3 OF 4)

			SHEET NO:	
DESIGNED BY: MTB	DATE:	NOVEMBER 2010	22	
CHECKED BY: DST	PROJECT NO:	051020	JZ	OF

# **GENERAL NOTES** (CONT.)

# DREDGING

ALL MATERIAL SHALL BE REMOVED BY THE CONTRACTOR WITHIN THE DREDGING PRISM SHOWN ON THE DRAWINGS, MATERIAL SHALL BE DREDGED USING SUCTION DREDGE, CLAMSHELL, BACKHOE OR OTHER APPROVED EQUIPMENT. ANY SUBMERGED OR FLOATING DEBRIS WILL BE REMOVED CONCURRENTLY WITH DREDGING AND DISPOSAL. THE DREDGED MATERIAL WILL BE DISPOSED OF AT AN OWNER APPROVED LOCATION. HAZARDOUS MATERIAL/WASTE SHALL BE DISPOSED OF IN ACCORDANCE WITH APPLICABLE STATE AND FEDERAL REGULATIONS. WHEN SUCH MATERIALS/WASTE ARE ENCOUNTERED THE CONTRACTOR WILL IMMEDIATELY NOTIFY THE OWNER.

CARE SHALL BE TAKEN WHEN DREDGING NOT TO HIT THE SHEET PILE OR OTHER STRUCTURAL COMPONENTS, DURING DREDGING OR TO EXCAVATE DEEPER THAN THE ALLOWABLE OVER-DEPTH SHOWN ON THE DRAWINGS. ANY DAMAGE TO THE STRUCTURE, WHICH IS CAUSED BY THE CONTRACTOR'S OPERATIONS, AS DETERMINED BY OWNER SHALL IMMEDIATELY BE REPAIRED TO THE PRE-DAMAGE CONDITION AT THE CONTRACTOR'S EXPENSE

A PRE-DREDGE SURVEY SHALL BE COMPLETED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL A MINIMUM OF 2 WEEKS PRIOR TO COMMENCING WITH THE WORK. THE PRE-DREDGE SURVEY SHALL ALSO INCLUDE QUANTITY CALCULATIONS FOR POTENTIAL DREDGE VOLUMES. THE ENGINEER WILL VERIFY THE CALCULATED VOLUMES PRIOR TO COMMENCEMENT OF DREDGING OPERATIONS. A POST-DREDGE SURVEY SHALL BE BASED ON A 25-FT GRID AND BE COMPLETED AND SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO FINAL PAYMENT. PROGRESS SURVEYS SHALL BE SUBMITTED. ALL SURVEY SUBMISSIONS SHALL INCLUDE A HARD COPY AND AN ELECTRONIC COPY OF THE DIGITAL SURVEY

A 1-FOOT ALLOWABLE OVER-DEPTH IS THE DREDGING TOLERANCE ALLOWED TO ENSURE REMOVAL OF ALL MATERIAL WITHIN THE DREDGING LIMITS. MATERIAL REMOVED FROM WITHIN THE 1-FOOT ALLOWABLE OVER-DEPTH WILL BE INCLUDED IN THE PAY QUANTITIES. DREDGING BEYOND THE 1-FOOT ALLOWABLE OVER-DEPTH WILL NOT BE PAID FOR. NO OVER-DEPTH ALLOWANCE IS PERMITTED BELOW THE SIDE SLOPES.

NO LATER THAN 14 DAYS PRIOR TO THE SCHEDULED START OF DREDGING AND DISPOSAL OPERATIONS, THE CONTRACTOR SHALL SUBMIT TO OWNER, FOR REVIEW AND APPROVAL, A DETAILED DREDGING AND DISPOSAL WORK PLAN, RIGHT OF WAY WORK PLAN AND QUALITY CONTROL PLAN. THE PLANS SHALL INCLUDE THE FOLLOWING:

- 1. ORDER IN WHICH THE WORK IS TO BE PERFORMED INDICATING THE WORK SEQUENCE, EQUIPMENT TO BE USED, METHODS OF OPERATION AND THE TIME REQUIRED TO COMPLETE EACH ACTIVITY. A LIST OF KEY PERSONNEL AND SUPERVISORY CHAIN WILL BE INCLUDED
- 2. PROCEDURES AND EQUIPMENT TO BE USED FOR DREDGING AND DISPOSAL OF DREDGED MATERIALS, INCLUDING NAME AND/OR NUMBER OF EQUIPMENT AND ITS CAPACITY, DUMPING SCHEDULE, HOURS OF OPERATION AND NAMES OF OPERATORS.
- 3. PROCEDURES AND EQUIPMENT FOR: COORDINATING AND PERFORMING HYDROGRAPHIC SURVEYS; POSITIONING DUMP BARGES AT THE DISPOSAL SITE AND ENVIRONMENTAL MONITORING, INCLUDING PROCEDURES FOR EMERGENCY SPILL CONTAINMENT AND REMOVAL OPERATIONS.
- 4. PROCEDURES AND EQUIPMENT FOR COLLECTING AND DISPOSING OF SUBMERGED AND FLOATING DEBRIS AS A RESULT OF DREDGING AND DISPOSAL OPERATIONS.
- 5. THE CONTRACTOR'S SUPERINTENDENT AND SITE POSITIONING SUPERVISOR AND THE OWNER REPRESENTATIVE SHALL ATTEND A PRE DISPOSAL CONFERENCE TO REVIEW THE WORK PLAN AND QUALITY CONTROL PROCEDURES FOR DISPOSAL POSITIONING ..

THE POST-DREDGE SURVEY MUST DEMONSTRATE THAT ALL MATERIAL HAS BEEN REMOVED FROM WITHIN THE DREDGE PRISM INDICATED ON THE DRAWINGS. THE POST-DREDGE SURVEY MUST DEMONSTRATE THAT ALL AREAS WITHIN THE LIMITS IDENTIFIED IN THE PLANS ARE AT OR BELOW THE REQUIRED DREDGE ELEVATION, AND THAT THE SIDE SLOPE IS NO STEEPER THAN INDICATED ON THE DRAWINGS. TO RECEIVE APPROVAL FOR PAYMENT, THE POST-DREDGE SURVEY AND FINAL QUANTITY CALCULATIONS SHALL BE SUBMITTED TO THE OWNER.

# DREDGE AREA SWEEP

DREDGED BASIN SHALL BE SWEPT PRIOR TO FINAL APPROVAL TO VERIFY THE COMPLETE ABSENCE OF HIGH SPOTS AND THAT THE BOTTOM IS RELATIVELY SMOOTH AND FREE OF ROCK PROTRUSIONS AND POINTS. THE OWNER SHALL BE PRESENT TO OBSERVE. THE DREDGED AREA SHALL BE SWEPT USING AT LEAST ONE OF THREE METHODS:

- 1. SIDE-SCAN SONAR AND/OR MULTI-BEAM HYDROGRAPHIC SURVEY TECHNOLOGY
- 2. SWEEP THE DREDGE BASIN WITH A STEEL PIPE, 20 TO 40 FEET LONG, 10" TO 14" DIAMETER, 0.5" MIN. WALL THICKNESS. THE STEEL PIPE SHALL BE SUSPENDED AT A PRESET DEPTH AND TOWED BEHIND A VESSEL. IF THE PIPE DOES NOT HIT ANYTHING THEN IT IS KNOWN THAT THE AREA IS FREE OF OBSTRUCTIONS AT THAT DEPTH
- 3. WIRE-DRAGGING: WIRE DRAGGING IS DONE BY ATTACHING A STEEL CABLE OR CHAIN TO TWO VESSELS, AND TOWED BETWEEN THEM AT A PRESET DEPTH. IF IT SNAGS, THEN THE OBSTRUCTION IS LOCATED. THE WIRE IS SUSPENDED AT A KNOWN DEPTH USING MULTIPLE BUOYS. IF THE CABLE RUNS FREELY ACROSS AN AREA, THEN IT IS KNOWN THAT THAT AREA IS FREE OF OBSTRUCTIONS TO THAT DEPTH. A DESCRIPTION OF WIRE DRAGGING IS AT THE FOLLOWING WEB SITE: <WWW.NJSCUBA.NET/ARTIFACTS/MISC\_WIRE\_DRAG.HTML>

### MISPLACED MATERIALS

OF MISPLACE MATERIALS DURING THE EXECUTION OF THE WORK, LOSE, DUMP, THROW OVERBOARD, SINK OR MISPLACE ANY DREDGE MATERIAL, DREDGE, BARGE, MACHINERY, APPLIANCE, OR OTHER MATERIALS, THE CONTRACTOR SHALL PROMPTLY RECOVER AND REMOVE THE SAME.

### DISPOSAL OF MATERIALS

ALL UNSUITABLE MATERIAL THAT CANNOT BE REUSED WITHIN THE LIMITS OF THE PROJECT SHALL BE DISPOSED OF AT THE DISPOSAL LOCATION. HAZARDOUS MATERIAL/WASTE SUCH AS AND BATTERIES SHALL BE DISPOSED OF IN ACCORDANCE WITH APPLICABLE STATE AND FEDÉRAL REGULATIONS AT NO COST TO THE OWNER. WHEN SUCH MATERIALS/ WASTE ARE ENCOUNTERED THE CONTRACTOR WILL IMMEDIATELY NOTIFY THE OWNER

# EXISTING STRUCTURES

THE CONTRACTOR SHALL REMOVE IN THEIR ENTIRETY THE STRUCTURES SHOWN ON THE PLANS.

### TBT CONTAMINATED SOIL

DURING THE GEOTECHNICAL INVESTIGATION FOR THIS PROJECT LABORATORY ANALYSIS OF SOIL SAMPLES INDICATED THAT TRIBUTYLTIN (TBT) WAS PRESENT IN THE INSITU SOIL SURROUNDING THE EXISTING BOAT

DREDGE MATERIAL FROM THE TOP TWO FEET OF MATERIAL IN THE TBT CONTAMINATED SOIL AREA SHALL BE DISPOSED OF AT A SUITABLE OFFSITE LOCATION. MATERIAL IN THIS AREA SHALL BE DREDGED WITHIN THE SITE IS NATURALLY DEWATERED TO THE GREATEST EXTENT POSSIBLE (I.E. LOW TIDE) TO MINIMIZE RE-SUPENSION OF TBT INTO THE WATER COLUMN.

# SILT CURTAIN

A SILT CURTAIN WHICH EXTENDS FROM WATER SURFACE TO SEA BOTTOM SHALL BE INSTALLED AND MAINTAINED ENCLOSING ALL DREDGING ACTIVITIES. PRIOR TO AND DURING DREDGING THE CURTAIN SHALL REMAIN IN PLACE UNTIL ALL DREDGING IS COMPLETE. CARE SHALL BE TAKEN TO MAINTAIN THE CURTAIN IN GOOD WORKING ORDER AND TO PROTECT THE CURTAIN FROM CONFLICTS WITH VESSELS USING THE HARBOR. SUBMIT SILT CURTAIN DETAILS AND MAINTENANCE AND OPERATION PLAN FOR REVIEW AND APPROVAL. SEE USACOE PERMIT FOR ADDITIONAL REQUIREMENTS.

# SUBMITTALS

THE ENGINEER'S REVIEW OF SUBMITTALS WILL BE FOR GENERAL CONFORMANCE ONLY AND IT SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR TO CONFORM TO ALL REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. ANY INTENDED DEVIATION FROM THE PLANS AND SPECIFICATIONS MUST BE SPECIFICALLY IDENTIFIED BY THE CONTRACTOR AND SPECIFICALLY APPROVED BY THE ENGINEER TO BE ACCEPTABLE

THE CONTRACTOR SHALL FULLY REVIEW AND STAMP SHOP DRAWINGS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS PRIOR TO SUBMITTING DRAWINGS TO THE ENGINEER. SHOP DRAWINGS OF ALL FABRICATED MATERIALS SHALL BE SUBMITTED TO THE ENGINEER FOR WRITTEN APPROVAL PRIOR TO FABRICATION OR MOBILIZATION OF ANY ITEM. A MINIMUM OF THREE SETS SHALL BE PROVIDED FOR EACH SUBMITTAL, OF WHICH TWO WILL BE RETURNED TO THE CONTRACTOR. THE CONTRACTOR SHOULD ALLOW TWO WEEKS FROM THE TIME OF RECEIPT FOR REVIEW OF SUBMITTALS BY THE ENGINEER FOR A REASONABLE NUMBER OF DRAWINGS. THE CONTRACTOR AND FABRICATOR ARE RESPONSIBLE FOR PROVIDING SHOP DRAWINGS THAT ACCURATELY SHOW THE APPROPRIATE DETAILS, DIMENSIONS, ASSEMBLY, MATERIAL REQUIREMENTS, AND OTHER REQUIREMENTS NECESSARY TO FABRICATE AND ERECT COMPONENTS OF THE STRUCTURE IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. AT THE JUDGMENT OF THE ENGINEER, SHOP DRAWINGS SHALL BE REJECTED WHEN THEY EITHER DEVIATE SIGNIFICANTLY FROM THE CONTRACT REQUIREMENTS WITHOUT THE ENGINEER'S PRIOR APPROVAL, OR ARE UNACCEPTABLE DUE TO INCOMPLETENESS, LEGIBILITY, OR NUMBER OF ERRORS

CERTIFICATIONS, MANUFACTURER'S DATA AND OTHER INFORMATION FOR ALL MATERIALS, INCLUDING THOSE NOT SPECIFICALLY SHOWN IN THESE NOTES OR ON INDIVIDUAL DRAWINGS, SHALL BE SUBMITTED TO THE ENGINEER FOR WRITTEN APPROVAL TO VERIFY CONFORMANCE WITH THE PLANS AND SPECIFICATIONS. IN THE EVENT THAT THE PLANS OR SPECIFICATIONS DO NOT SPECIFICALLY REFERENCE A MATERIAL, THE APPROVAL OF MATERIALS WILL BE BASED ON ITS CONFORMANCE TO THE INTERNATIONAL BUILDING CODE. ALL METHODS AND MATERIALS SHALL CONFORM TO THESE GENERAL NOTES, GOOD WORKMANSHIP, GENERALLY ACCEPTED INDUSTRY STANDARDS, AND MANUFACTURER'S RECOMMENDATIONS.

WORK PERFORMED BY THE CONTRACTOR PRIOR TO RECEIVING OWNER/ENGINEERS WRITTEN APPROVAL OF REQUIRED SUBMITTALS SHALL BE AT THE CONTRACTORS OWN RISK. ANY SUCH WORK REQUIRED BY THE OWNER/ENGINEER TO BE REMOVED AND/OR REPLACED SHALL BE AT THE EXPENSE OF THE CONTRACTOR AT NO EXPENSE TO THE OWNER.

THE FOLLOWING IS A PARTIAL LIST OF REQUIRED SUBMITTALS FOR THIS PROJECT. THIS DOES NOT CONSTITUTE A COMPLETE LIST AS IT WILL VARY DEPENDING UPON THE CONTRACTOR'S METHODS.

CONSTRUCTION PLANS (INCLUDES PLAN DRAWINGS AND WRITTEN DESCRIPTION OF METHODS): SURVEY PLAN AND UPDATES DEMOLITION PLAN GENERAL WORK PLAN SEQUENCING BY FLOAT DESIGNATION STAGING AREA PLAN - INCLUDING DATES OF USE, SECURITY AND PEDESTRIAN CONTROL PLAN PILE DRIVING PLAN AND EQUIPMENT FLOATATION BILLET INSTALLATION PLAN DREDGING, EXCAVATION WORK PLAN CROSS SECTIONS AND QUANTITY COMPUTATIONS SILT CURTAIN OPERATION PLAN 10. DETAILED CONSTRUCTION SCHEDULE 11. HARBOR MASTER'S BUILDING RELOCATION PLAN 12 TRAFFIC CONTROL PLAN 13. EXISTING FLOAT TEMPORARY ACCESS & UTILITY PLAN/NOTIFICATION SHOP DRAWINGS AND MATERIAL CERTIFICATION STRUCTURAL STEEL SHOP DRAWINGS STEEL PIPE PILES

GALVANIZING CERTIFICATION AND/OR METALLIZING CERTIFICATION METALLIZING REPAIR METHOD AND MATERIALS AWS WELDING CERTIFICATION FOR ALL WELDERS UTILIZED ON THE PROJECT WELDING PROCEDURES FOR ALL SHOP AND FIELD WELDS STEEL FARRICATION DRAWINGS TIMBER FLOAT SHOP DRAWINGS TIMBER GRADING AND PRESSURE TREATMENT CERTIFICATIONS 10. STEEL PILE SHOES 11. SIGN SHOP DRAWINGS 12. UHMW SHOP DRAWINGS 13. RUBBER PIPE HINGE BUSHING & COMPLETE SAMPLE w/ UHMW BUSHING 14. CONCRETE MIX DESIGN & TEST RESULTS 15 REINFORCING STEEL 16. TRACTION PLATE 17. POLYSTYRENE SHOP DRAWINGS & CUT SHEETS 18. RED-LINED AS-BUILT DRAWINGS

TIMBER FLOAT SHOP DRAWINGS SHALL PICTORIALLY DEMONSTRATE ALL ELEMENTS (TIMBERS, STEEL HARDWARE AND BOLTS, AND FOAM) THAT MAKE UP EACH TYPE OF FLOAT MODULE AND THE POSITION OF THE ELEMENTS ON THE ELOAT.

# AS-BUILT PLANS

OWNER/ENGINEER.

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PATENT - US 6,715,964 B2 PATENT - US 7,018,141 B2 PATENT - US 7,488,140 B2 REV DATE DESCRIPTION	THE OPEN CELL SYSTEM IS PATENTED			
PATENT - US 7,018,141 B2 PATENT - US 7,488,140 B2  REV DATE DESCRIPTION	PATENT – US 6,715,964 B2			
PATENT - US 7,488,140 B2	PATENT – US 7,018,141 B2			
	PATENT – US 7,488,140 B2	REV	DATE	DESCRIPTION

PND Engineers Inc. is not responsible for safety

	1506 West 36th Avenue
	Anchorage, Alaska 99503
	Phone: 907.561.1011
ENG	East 907 563 4220

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THE CONTRACTOR SHALL MAINTAIN A SET OF AS-BUILT PLANS IN THE ONSITE PROJECT OFFICE. THE AS-BUILT PLANS SHALL BE KEPT UP TO DATE THROUGHOUT THE PROJECT WITH THE LATEST AS-BUILT DIMENSIONS AND DETAILS AS APPROVED BY THE ENGINEER AND SHALL BE SUBMITTED TO THE OWNER AT THE END OF THE PROJECT. FINAL PROJECT PAYMENT SHALL NOT BE MADE TO THE CONTRACTOR UNTIL RED-LINED AS-BUILT DRAWINGS HAVE BEEN SUBMITTED BY THE CONTRACTOR AND APPROVED BY THE

# DRAFT **ISSUED FOR BID** 11/11/10

DESIGNED BY:

CHECKED BY:

![](_page_97_Picture_47.jpeg)

# WHITTIER SMALL BOAT HARBOR **IMPROVEMENTS - PHASE I**

**GENERAL NOTES** (4 OF 4)

			SHEET NO:	
MTB	DATE:	NOVEMBER 2010	22	
DST	PROJECT NO:	051020	55	OF 🕻